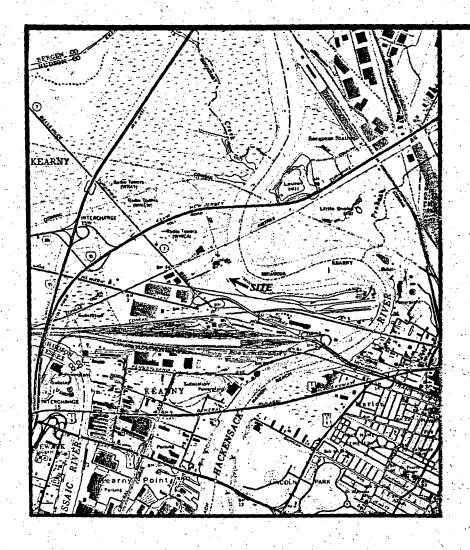


## SUPPLEMENTAL REM.

## STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY



250559

APR | 2 1999

Prepared For:
Standard Chlorine Chemical Company
1035 Belleville Turnpike
Kearny, New Jersey 07032

Prepared By:
Key Environmental, Inc.
1200 Arch Street, Suite 200
Carnegie, Pennsylvania 15106

## TABLE OF CONTENTS

LIST	OF TA	BLES	ii
LIST	OF API	PENDICES	iii
LIST	OF AB	BREVIATIONS AND ACRONYMS	iv
	1.0		
1.0	INTRO	DUCTION	1 - 1
	1.1	REPORT ORGANIZATION	1 - 2
	1.2	SILL DACKOROUND	1-2
		1.2.1 Site Description	1 - 2
	• • • • • • •	1.2.2 Property Use	
		1.2.3 Topography and Surface Drainage	
		1.2.4 Site Geology	
		1.2.5 Site Hydrogeology	1 - 4
			4.134
2.0		LEMENTAL REMEDIAL INVESTIGATION SCOPE OF WORK	
			2 - 1
	2.1	TRANSFORMER AREA SURFACE SOIL/CONCRETE CHIP SAMPLING	•
		DRODUCTION WELL CANDUNG AND AND ON CENT	
	2.2	PRODUCTION WELL SAMPLING AND ABANDONMENT	2 - 2
*	2.3	SOIL SAMPLING AND ANALYSIS	2 - 4
	2.4 2.5	MONITORING WELL INSTALLATION GROUNDWATER SAMPLING	2-3
	2.5 2.6	POSTIM INVESTIGATION	2 - 0
	2.0	ROST™ INVESTIGATION DNAPL RECOVERY EVALUATION	2 - 8
	2.7	SITE SURVEY	
	2.0		2-0
 3-0 S	RIPPLE	MENTAL REMEDIAL INVESTIGATION RESULTS	3 - 1
	3.1	TRANSFORMER AREA SOIL SAMPLING AND ANALYSIS	
	, .	MW-3L AREA SOIL SAMPLING AND ANALYSIS	
	3.3	GROUNDWATER SAMPLING AND ANALYSIS	
: ·	3.4	GROUNDWATER FLOW	3 - 3
	3.5	ROST™ SURVEY RESULTS	3 - 3
		3.5.1 DNAPL Distribution in the Fill Unit	3 - 4
		3.5.2 DNAPL Distribution in the Sand Unit	3 - 5
		3.5.2 DNAPL Distribution in the Sand Unit	3 - 5
	3.6	DNAPL RECOVERY EVALUATION	3 - 6
	4.5		

4 A	CONICT TISTONS	S AND RECOMM	FNDATIONS	 	1 _ 1
4.0	CONCLOSIONS	S WIND ICT COMMI	FLIDITIONS	 	7 7 7

## LIST OF TABLES

TABLE	TITLE
2-1A	FORMER PRODUCTION WELL - GROUNDWATER
	ANALYTICAL RESULTS FOR VOLATILE ORGANIC
	COMPOUNDS
2-1B	FORMER PRODUCTION WELL - GROUNDWATER
	ANALYTICAL RESULTS FOR SEMI-VOLATILE ORGANIC
	COMPOUNDS
2-1C	FORMER PRODUCTION WELL - GROUNDWATER
	ANALYTICAL RESULTS FOR METALS
3-1	SOIL ANALYTICAL RESULTS FOR PCB'S
3-2	SOIL ANALYTICAL RESULTS - SEMI-VOLATILE ORGANIC
	COMPOUNDS
3-3	GROUNDWATER ANALYTICAL RESULTS
3-4	POTENTIOMETRIC SURFACE MEASUREMENTS

## LIST OF FIGURES

FIGURE	A REPORT OF THE PROPERTY OF TH
1-1	SITE LOCATION MAP
1-2	SITE MAP
2-1	PCB SOIL SAMPLE LOCATION MAP
2-2	SOIL BORING AND MONITORING WELL LOCATION MAP
2-3	CPT/ROST™ SOUNDING LOCATION MAP
3-1	POTENTIOMETRIC SURFACE MAP - FILL UNIT
3-2	POTENTIOMETRIC SURFACE MAP - SAND UNIT
3-3	TYPICAL MULTI-WAVELENGTH (MWL) SIGNATURES
3-4	INFERRED DNAPL DISTRIBUTION IN FILL UNIT
3-5	INFERRED DNAPL DISTRIBUTION IN SAND UNIT
3-6	CONTOUR MAP - TOP OF MEADOW MAT ELEVATION

April 1999	Kearny, New Jersey
3-7	CONTOUR MAP - TOP OF VARVED CLAY ELEVATION
3-8	GEOLOGIC CROSS SECTION LOCATION MAP
3-9A	GEOLOGIC CROSS SECTION A-A'
3-9B	GEOLOGIC CROSS SECTION B-B'
3-9C	GEOLOGIC CROSS SECTION C-C'
	하는 사람들은 사람들이 되었다. 그는 사람들은 사람들이 되었다. 그는 사람들은 사람들은 사람들은 사람들은 사람들은 사람들이 되었다. 
	LIST OF APPENDICES
APPENDIX	TITLE
A	BORING LOGS AND MONITORING WELL CONSTRUCTION
	DIAGRAMS
В	ROST™ REPORT
C	SURVEY DATA
D	DNAPI RECOVERY DATA

And Conference of the Conferen

The second of th

## LIST OF ABBREVIATIONS AND ACRONYMS

ACO Administrative Consent Order bis(2-ethylhexyl)phthalate

COPR Chromium Ore Processing Residue

CPT Cone Penetrometer Test

DNAPL Dense Non Aqueous Phase Liquid
DOT Department of Transportation

ERM Environmental Resources Management, Inc.

FRI Focused Remedial Investigation

ft feet

ft-msl feet above mean sea level ft-bgs feet below ground surface Fugro Geosciences, Inc.

GWQC Ground Water Quality Criteria

HMDC Hackensack Meadowlands Development Commission

HSA Hollow Stem Auger ID Inner Diameter

IGWSCC Impact to Groundwater Soil Cleanup Criteria

in Inches

JCA James C. Anderson and Associates

KEY Key Environmental, Inc.

Koppers Company, Inc.

μg/kg micrograms per kilogram

μg/L micrograms per liter

MW Monitoring Well

NAD83 North American Datum of 1983 NAPL Non Aqueous Phase Liquid

NAVD88 North American Vertical Datum of 1988

NJDEP New Jersey Department of Environmental Protection NRDCSCC Non-Residential Direct Contact Soil Cleanup Criteria

nm nano-meters

OVM Organic Vapor Monitor
PCBs Polychlorinated biphenyls
PID Photo-ionization Detector

ppb parts per billion ppm parts per million

PRAP	Proposed Remedial Action Plan
RASR	Remedial Action Selection Report
RAWP	Remedial Action Work Plan
RDCSCC	Residential Direct Contact Soil Cleanup Criteria
ROSTTM	Rapid Optical Screening Tool
SCCC	Standard Chlorine Chemical Company
SNP	Standard Naphthalene Products
SRI	Supplemental Remedial Investigation
STL	Severn Trent Laboratories
SVOCs	Semi-volatile Organic Compounds
TAL	Target Analyte List
TCL	Target Compound List
VOCs	Volatile Organic Compounds
Weston	Roy F. Weston, Inc.
WTM	Wavelength Time Matrix

#### 1.0 INTRODUCTION

This report presents the results of the Supplemental Remedial Investigation (SRI) conducted at the Standard Chlorine Chemical Company. Inc. (SCCC) Site located in Kearny, New Jersey. The investigation of the SCCC Site is being performed pursuant to the Administrative Consent Order (ACO) executed on October 20, 1989 between SCCC and the New Jersey Department of Environmental Protection (NJDEP).

The objectives of the SRI were to complete characterization of soil and groundwater over the western two-thirds of the SCCC property and to further evaluate the nature and extent of potentially recoverable Dense Non-Aqueous Phase Liquid (DNAPL) at the Site. Specific items to be addressed during the SRI were identified in the NJDEP November 8, 1995 comments on the Remedial Investigation Report dated May 1993. These items included the following:

- Addressing septic tanks 4 and 5, including potentially contaminated surrounding soils, as source areas to be addressed in accordance with the NJDEP Technical Requirements for Site Remediation (N.J.A.C. 7:26E);
- Delineation of the extent of polychlorinated biphenyls (PCBs) in soils surrounding the transformer pad;
- Determination of the source and extent of dichlorobenzene and phenol detected in the groundwater sample collected from monitoring well MW-3L; and,
- Determination of the source and extent of free product in the area of monitoring well MW-15L.

In addition to these items, SRI activities included the sampling and abandonment of the inactive former plant production well.

As indicated in the NJDEP November 8, 1995 comment letter, these issues were rendered "dormant" while SCCC completed a Focused Remedial Investigation (FRI) and prepared a Proposed Remedial Action Plan (PRAP) for the eastern portion of the facility. The PRAP and FRI Report were submitted to the NJDEP in January 1997. In a letter dated June 3, 1997, NJDEP conditionally approved the proposed capping and containment remedy for the eastern portion of the Site, as presented in the PRAP, and conditionally accepted the FRI Report. However, the NJDEP did not accept SCCC's proposal for passive DNAPL collection. The NJDEP directed SCCC to address the comments on the PRAP in the Remedial Action Work Plan (RAWP).

A meeting was held between SCCC and NJDEP on August 7, 1997 to discuss the investigation and remediation of the SCCC Site. At the meeting, the parties agreed to proceed with the remaining investigative activities stipulated in the NJDEP November 8, 1995 comment letter. On September 10, 1997, SCCC provided NJDEP with a letter plan for implementation of the SRI. Following two rounds of comments and responses, SCCC and NJDEP met on February 4, 1998 to reach a consensus on the outstanding investigation issues. By letter dated February 18, 1998, NJDEP indicated that the resolution of issues during the February 4, 1998 meeting would allow SCCC to proceed with implementation of the SRI.

#### 1.1 REPORT ORGANIZATION

Section 1.0 of this report includes this introduction, and an overview of site background information. Section 2.0 summarizes the SRI scope of work and the methodologies used to implement this SRI. Section 3.0 discusses the results of this SRI. Section 4.0 presents conclusions and recommendations.

Boring logs and monitoring well construction diagrams are attached as Appendix A. Cone Penetrometer Test (CPT) and Rapid Optical Screening Tool (ROST<sup>TM</sup>) data are included as Appendix B. Appendix C contains survey information for the monitoring wells soil borings. CPT/ROST<sup>TM</sup> soundings and other select Site features. The data acquired during the DNAPL recovery evaluation performed during the SRI is included in Appendix D. One copy of the laboratory data packages has been submitted to the NJDEP under separate cover.

#### 1.2 SITE BACKGROUND

The following subsections present an overview of site background information. Much of this information was gathered from either the 1993 RI Report prepared by Roy F. Weston, Inc. (Weston) or, the 1997 FRI Report prepared by Environmental Resources Management, Inc. (ERM).

#### 1.2.1 Site Description

The SCCC Site is approximately 25 acres in area and is located at 1015 through 1035 Belleville Turnpike, Kearny New Jersey. The Site location is shown on Figure 1-1. The Site is bounded by the Hackensack River to the east, Belleville Turnpike to the west, property owned by Maxus Energy (formerly Diamond Shamrock Company, Inc.) to the north, and the former Koppers Company, Inc. (Koppers) Seaboard Site to the south. Railroad tracks were historically present at the southwestern corner of the Site along the property boundary. A north trending rail spur was historically located west of the lagoon area. The Site is comprised of several parcels. The general Site layout is shown on Figure 1-2.

A detailed description of the Site operational history is provided in the RI Report prepared by Weston. In summary, operations conducted at the Site by various entities included a naphthalene refining process and manufacture of products from dichlorobenzene and trichlorobenzene. The naphthalene refining operation was conducted in the eastern two-thirds of the Site. The manufacture of dichlorobenzene and trichlorobenzene products occurred on the western one-third of the property.

Residual waste materials, comprised of sludges and oils, were placed, and are currently present, within a lagoon located in the eastern portion of the property. According to the Weston RI Report, the lagoon occupies an area of approximately 33,000 square feet and has an average depth of 6 feet. Thus, the lagoon contains approximately 7,300 cubic yards of material.

## 1.2.2 Property Use

Currently no industrial operations are conducted at the Site. The Site is zoned by the Hackensack Meadowlands Development Commission (HMDC) as heavy industrial. Present Site activities are limited to use of the office building on the western portion of the Site. The Site and surrounding properties are currently used for non-residential purposes. There are no residential developments in the vicinity of the Site and it is highly probable that the future uses of the SCCC Site and surrounding properties will remain non-residential in accordance with HMDC zoning.

## 1.2.3 Topography and Surface Drainage

The ground surface at the Site is relatively flat, primarily ranging in elevation from 3 to 8 feet above mean sea level (ft-msl). The highest surface elevation, approximately 10-ft msl exists in the southeast corner of the Site. The eastern and western portions of the Site generally slope to a central drainage swale. This swale directs water to the south and then to the east along the southern property boundary for discharge to the Hackensack River via the south outfall. In addition to on-Site drainage, this ditch receives some sheet flow run-off from off-Site commercial and industrial properties to the west and south of the Site. The south outfall is equipped with a tide gate to prevent backflow from the Hackensack River during high tide.

A 24-inch diameter underground concrete stormwater pipe is present along the northern property boundary of the Site. This pipeline receives run-off via drop inlets from the Maxus property to the north of the SCCC property, as well as drainage from off-Site commercial, and industrial properties to the west. The stormwater pipe discharges to Hackensack River through an outfall which is located north of the Site. The outfall is equipped with a tide gate to prevent backflow from the river during high tide.

The Hackensack River is adjacent to the entire eastern property boundary. The Hackensack River is tidally influenced and discharges to Newark Bay to the south. The overall direction of flow in the Hackensack River adjacent to the Site is from north to south. The Hackensack River in the vicinity of the Site receives some sheet flow run-off from the SCCC property, stormwater discharge from the southern drainage ditch, run-off and related discharges from properties to the north of SCCC via the north stormwater pipe, and surface water from downstream when the direction of flow reverses during high tide.

#### 1.2.4 Site Geology

The Site geology consists of coastal plain sediments overlaying Triassic-age bedrock. Prior to industrial development, this area consisted of marshlands that bordered the Hackensack River. Fill materials were placed in the coastal marshlands of this region to create property for industrial development. These fill materials generally consisted of chromium ore processing residue (COPR) and silty sand. According to the Weston RI report. COPR was placed as fill on approximately 85% of the SCCC Site to depths ranging between 2 and 10 feet below the present grade. The original marsh surface, now located beneath the fill materials, consists of silt, humus and peat. This layer is regionally referred to as the Meadow Mat and is typically 2 to 5 feet thick at the SCCC Site. The upper surface of the Meadow Mat is undulating rather than planar.

A sand layer is present beneath the Meadow Mat that is generally less than 10 feet thick. A varved clay unit is present beneath the sand layer. This unit is continuous beneath the Site. The thickness of this unit is estimated at greater than 40 feet based on subsurface data acquired at the adjacent Koppers Site. A glacial till is present beneath the varved silt and clay layer. Based on information acquired from the Koppers Site, bedrock in the area is composed of red shale and sandstone of the Triassic-age Brunswick Formation, and ranges in depth from 60 to 100 feet below ground surface.

#### 1.2.5 Site Hydrogeology

The water table at the Site occurs in the fill material placed above the Meadow Mat. The Meadow Mat acts as a basal semi-confining unit that limits, but does not completely eliminate, the hydraulic connection between the shallow fill materials and the deeper sand layer. Data acquired at nested well locations indicate a downward vertical gradient exists between the fill unit and the underlying sand layer. Groundwater in the fill unit exists under unconfined conditions. Previous studies have indicated that the fill unit is not tidally influenced.

A potentiometric mound exists in the fill unit in the vicinity of the lagoon. Groundwater flows radially away from this potentiometric high in the lagoon area. Beyond the influence of this mound.

groundwater flow in the fill unit is primarily to the south, approximately parallel to the direction of flow in the Hackensack River.

Groundwater in the sand unit beneath the Meadow Mat exists under semi-confined conditions. The underlying varved clay acts as an effective barrier to the downward migration of groundwater from this unit. Groundwater flow in the sand unit is primarily to the south-southeast. A slight potentiometric mound has been observed in the area to the northwest of the lagoon. Groundwater flow toward the east is observed in the northeast portion of the Site in the immediate vicinity of this mound. Horizontal hydraulic gradients in the sand unit are relatively flat ranging from 0.003 to 0.008. The average horizontal hydraulic conductivity in the sand unit, as determined through slug tests, is 5.34 ft/day (1.9x10<sup>-3</sup> cm/sec).

Groundwater within the sand unit is tidally influenced to some extent. Fluctuations in potentiometric surface elevations that are correlated to tides in the Hackensack River have been observed in monitoring wells located immediately adjacent to the river. Because of its limited extent, the tidal influence has not been observed to create significant changes in groundwater flow directions between high and low tide.

# 2.0 SUPPLEMENTAL REMEDIAL INVESTIGATION SCOPE OF WORK AND METHODOLOGIES

This section summarizes the scope of work completed during the SRI and the methodologies employed during implementation of the field activities. The following presents an overview of the activities performed:

- Collection of surface soil samples and a concrete chip sample at the location of the former transformer for analysis for PCBs;
- Sampling and abandonment of a former on-Site production well;
- Analysis of select soil samples from two soil borings completed in the vicinity of monitoring well MW-3L and the borings drilled for installation of monitoring wells MW-16L and MW-17L;
- Installation of two new groundwater monitoring wells in the vicinity of existing monitoring well MW-3L and the collection and analysis of groundwater samples from these new wells:
- Investigation of the extent of free-phase DNAPL in the shallow fill and underlying sand groundwater-bearing units: and,
- Evaluation of the feasibility of DNAPL recovery.

The collection of surface soil and concrete chip samples for PCB analysis and the sampling of the former production well were performed during an October 1998 mobilization. The remaining scope of work was completed during January 1999.

## 2.1 TRANSFORMER AREA SURFACE SOIL/CONCRETE CHIP SAMPLING

The purpose of this task was to further delineate the extent of PCBs detected in sediment in the transformer area during the 1993 RI. One concrete chip sample and three surface soil samples were collected from the former transformer pad area on October 9, 1998. PCB sample locations are shown on Figure 2-1.

The concrete chip sample was obtained from a 1-ft square area, which was located in front of the drain spigot of the existing transformer. All surface debris was removed from the area, and fine

materials were washed away from the area by rinsing with laboratory-supplied de-ionized water. An electric jack-hammer was used to create a starter hole in the concrete to a depth of approximately 1/2 inch. Subsequently, a masonry chisel and hammer were used to loosen concrete chips across the selected 1-ft square area to a depth of approximately 1/4-inch. The chips were collected and placed in the laboratory-supplied containers.

No unpaved surface exists in the vicinity of the transformer. The ground surface in this area is covered by asphalt. The soil samples were obtained from immediately beneath the asphalt in the area immediately to the north of the transformer pad. The jack-hammer was used to cut an approximately 1.5-ft square access hole through the asphalt pavement at three locations surrounding the concrete transformer pad. The pavement materials were removed from the hole, and the underlying soil materials were sampled. The soil samples were homogenized in a stainless-steel mixing bowl then placed in the sample container. Following sample collection, asphalt patch material was used to repair the pavement surface at each sampling location.

The concrete chip sampling equipment (masonry chisel) and shallow/surface soil sampling equipment (stainless-steel mixing bowl and scoop) were decontaminated after each use. Decontamination procedures included the following:

- Wash with tap-water and Alconox;
- Rinse with tap water:
- Rinse with pesticide-grade acetone; and,
- A final triple rinse of de-ionized water.

A field rinsate blank was collected from the decontaminated equipment following soil or concrete chip sampling activities. The field rinsate blank was collected by pouring laboratory supplied deionized water over the decontaminated chisel and scoop and into the stainless-steel mixing bowl. The rinsate was then poured from the bowl into the appropriate laboratory sample containers

The concrete chip and the soil samples were analyzed for the presence of PCBs by EPA SW846 Methods 8081 and 8082 by NJDEP-certified laboratories. Severn Trent Laboratories (STL) performed the Method 8081 analyses. The Method 8082 analyses were performed by Philip Analytical Services.

#### 2.2 PRODUCTION WELL SAMPLING AND ABANDONMENT

Sampling and abandonment were conducted in accordance with the December 30, 1996 Work Plan, as modified by the June 17, 1997 NJDEP letter. Prior to abandonment, groundwater samples were

collected from the 10-inch diameter, 368 foot deep production well on October 8, 1998 to provide data to support the selection of an appropriate method for management of any groundwater displaced during the grouting operation. In accordance with the NJDEP June 17, 1997 letter, the groundwater samples were collected from three discrete intervals representing depths of ten feet below the bottom of the surface casing (88 ft-bgs), the middle of the water column (228 ft-bgs), and the bottom of the well (360ft-bgs).

The three groundwater samples were collected using a Teflon® point source bailer attached to Teflon® coated stainless steel cable. Samples were transferred directly from the bailer to analytical laboratory prepared sample containers. The sample containers were immediately placed in an insulated cooler containing ice.

The bailer and cable were decontaminated between samples using potable water and Alconox wash: potable water rinse; pesticide grade acetone rinse; de-ionized water rinse; nitric acid rinse; and final triple rinse of de-ionized water. A rinsate blank was collected to examine the effectiveness of this decontamination procedure. A trip blank was prepared by the analytical laboratory and sent with the samples to examine for the possibility of sample contamination during shipping and storage.

The production well groundwater samples were analyzed for the presence of Target Compound List (TCL) volatile organic compounds (VOC), TCL semi-volatile organic compounds (SVOC) and Target Analyte List (TAL) metals. The VOC and SVOC analyses were performed using EPA Methods 624 and 625, respectively. The metals were analyzed using EPA 200 series methods. The production well groundwater samples were analyzed by STL.

The results of the groundwater analyses were provided to NJDEP on November 13. 1998 and are summarized in Tables 2-1A. 2-1B, and 2-1C. Based on the results of these analyses, NJDEP recommended that any groundwater produced during closure of the production well be discharged to the lagoon in the eastern portion of the Site. The NJDEP issued a New Jersey Pollutant Discharge Elimination System Permit-by-Rule authorizing the discharge of the water to the lagoon area on December 10, 1999.

Closure of the former production well was completed in January 1999 by James C. Anderson and Associates (JCA) of Moorestown, New Jersey. JCA is a New Jersey licensed well driller that is certified by the NJDEP to seal wells.

The former production well was abandoned by inserting 1.5-inch diameter flexible polypropylene pipe from surface to the bottom of the well, approximately 368-feet below ground surface. A cement-mixer truck was brought onto the Site for the purpose of providing cement grout for the

abandonment of the former production well. The cement grout was produced by mixing 24,440 pounds of cement with approximately 1.560 gallons of water. The grout was slowly poured via a trough into the drillers mud tub and then pumped through the 1.5 inch diameter tremie pipe that was inserted to the base of the production well. A second pump was used to remove the displaced groundwater from the casing and direct it to the lagoon area via polypropylene piping. The NJDEP well abandonment report form was submitted to the NJDEP Bureau of Water Allocation by JCA.

#### 2.3 SOIL SAMPLING AND ANALYSIS

As recommended by NJDEP in its November 8, 1995 letter, additional investigation was completed west and northwest of monitoring well MW-3L to determine if the dichlorobenzene and phenol detected in this well originate from a source in this area. As agreed between SCCC and NJDEP at the February 4, 1998 meeting, two soil borings (SB-15 and SB-16) were completed for this purpose. Soil boring SB-15 was located between the guardhouse on the northern property boundary and Building #1. Soil boring SB-16 was located near the western property boundary. The locations of the soil borings are shown on Figure 2-2.

The soil borings were completed from ground surface to the top of the varved clay layer. Drilling of the borings was accomplished with a Failing F-7 drilling rig. The two soil borings were completed through the use of 3-1/4-inch inside diameter (ID) hollow stem augers (HSA) and continuous split spoon sampling.

All soil samples were visually screened for evidence of potential constituent impact. Soil samples were also screened for the presence of organic vapors with an H-Nu photo-ionization detector (PID) or an Organic Vapor Monitor (OVM). No visual evidence of impact or elevated PID or OVM readings were observed in any of the soil samples. Both soil borings were tremie grouted using a cement/bentonite grout mixture immediately upon their completion. Soil boring logs are presented in Appendix A.

Three soil samples per boring were submitted to STL for laboratory analysis for SVOCs by EPA Method 8270. These soil samples were collected at the following depths:

- immediately beneath the 1-foot thick crushed stone surface layer;
- immediately above the meadow mat layer; and,
- immediately above the varved clay layer.

The purpose of collecting the near surface soil sample was to examine for the presence of a surficial source at these locations. The purpose of analyzing the sample collected from directly above the meadow mat was to examine the material for the potential presence of SVOCs within the fill unit.

KEA

The purpose of the soil sample collected from immediately above the varved clay was to examine for the potential presence of constituents migrating within the sand unit along the surface of the confining clay from an upslope source.

In addition, two soil samples were collected for laboratory analysis at each of the two new monitoring well locations (MW-16L and MW-17L). These soil samples were collected from the 0 to 2-foot interval and from the soils located immediately on top of the meadow mat layer. Soil samples were not collected at the top of the varved clay layer as the groundwater monitoring well was screened in this zone and the data obtained through the soil analyses would be redundant with the groundwater analyses conducted at these locations.

#### 2.4 MONITORING WELL INSTALLATION

Two groundwater monitoring wells (MW-16L and MW-17L) were installed within the sand unit in the western portion of the Site to define the extent of constituents in groundwater in the vicinity of monitoring well MW-3L. Groundwater monitoring well MW-16L was located northwest of MW-3L near the guardhouse on the western property boundary. Groundwater monitoring well MW-17L was located southeast of MW-3L near the southern property boundary. The locations of these monitoring wells are shown on Figure 2-2.

Borings for the monitoring well installation were completed using HSA and fluid rotary drilling techniques. Continuous split-spoon soil sampling was performed at both monitoring well locations to define the stratigraphy at the well locations. Hollow stem auger drilling methods were used from ground surface to an elevation below the top of the meadow mat. A 6-inch diameter steel casing was inserted into the aquitard and was grouted in place with a cement/bentonite mixture. The grout was allowed to cure for a minimum of 72-hours before the borehole was further advanced. The boring was then completed to the top of the varved clay using fluid rotary drilling techniques.

All groundwater monitoring wells were installed with the base of the screen located at an elevation slightly below the top of the varved clay layer to allow for detection of a DNAPL layer, if present. Groundwater monitoring well MW-16 was completed with a five foot long screen, whereas monitoring well MW-17L was constructed with a 3-foot long screen. The screen at location MW-17L was shorter due to the limited thickness of sand unit between the base of the meadow mat and the top of the varved clay at this location. A sand filter was installed between the well screen and the borehole wall to an elevation above the top of the well screen. Both monitoring wells were constructed with a 2-foot minimum bentonite seal located above the sand filter. The seal was constructed of bentonite pellets which were allowed to hydrate for a minimum of 30-minutes before continuing with the well construction. Cement/bentonite grout was then tremied from the top of the

KYY

bentonite to within 2-feet of ground surface. Concrete pads were constructed at the ground surface and were sloped away from the well to prevent surface water infiltration through the well. Flush mount covers with locking inner plugs were installed to secure the wells.

The two newly installed groundwater monitoring wells were developed on January 20, 1999 using a Whale submersible pump. Twenty and sixteen well casing volumes of groundwater were removed from monitoring wells MW-16L and MW-17L, respectively. Groundwater field parameters were monitored during the development process using a Horiba meter. These measurements included temperature, pH, salinity, turbidity, conductivity and dissolved oxygen. KEY field personnel noted the presence of an emulsified NAPL during the development of groundwater monitoring well MW-17L. The presence of the emulsified NAPL in the development fluids at location MW-17L prevented further monitoring of the groundwater quality parameters with the Horiba to prevent damage to this instrument.

#### 2.5 GROUNDWATER SAMPLING

Groundwater sample collection was performed on February 2. 1999 at locations MW-16L and MW-17L. Purging and sampling was performed using a peristaltic pump. Low flow sampling techniques were utilized to minimize sample turbidity. Dedicated tubing was used during groundwater sampling such that decontamination of field equipment was not necessary. The KEY field personnel noted that DNAPL was not encountered at location MW-17L during purging or sampling.

A field duplicate was collected at Well MW-17L. A field blank was also collected. All samples were shipped overnight to STL. The groundwater samples were analyzed for SVOC using EPA Method 625.

#### 2.6 ROST<sup>TM</sup> INVESTIGATION

The purpose of the ROST<sup>™</sup> investigation was to further delineate the extent of DNAPL in the fill material and underlying sand unit. A ROST<sup>™</sup>-equipped cone penetrometer system, that utilizes laser light to fluoresce hydrocarbon-based chemicals present in the subsurface, was used for this task. Fugro Geosciences, Inc. (Fugro) of Houston, Texas was subcontracted by KEY to provide this service.

The laser light is introduced to the subsurface soils through a sapphire window present on the side of the cone penetrometer. The fluorescence of the hydrocarbons in the subsurface is detected by a photoelectric cell and the signal is processed in a computer located in the cone penetrometer truck. The computer measures the intensity of the fluorescence and the wave spectrum of the signal. The

wave spectrum of the response is used to provide a qualitative assessment of the chemical composition of the observed NAPL. The wave spectral analysis is referred to as a wavelength time matrix (WTM). The WTM monitors four wavelengths at 340, 390, 440 and 490 nanometers (nm). The relative intensity of the fluorescence, when calibrated to the chemical composition of the NAPL can provide a qualitative estimate of the relative concentration of that NAPL in the subsurface.

Thirty-one ROST<sup>TM</sup> soundings were completed during this investigation. ROST<sup>TM</sup> locations are shown on Figure 2-3. The ROST<sup>TM</sup> testing was predominantly performed in the vicinity of the former lagoon located in the eastern area of the Site, and around Buildings 2, 3 and 4. Six additional ROST<sup>TM</sup> locations were added in the field to assist in the delineation of suspected NAPL in the subsurface. These locations were added between the former lagoon and Buildings 3 and 4. ROST<sup>TM</sup> sample locations were also added between Building #2 and the abandoned warehouse building. A ROST<sup>TM</sup> test was also performed adjacent to location SB-15 to examine the instrument response at a location where no NAPL was observed in the subsurface.

ROST<sup>TM</sup> data were acquired at approximately one-inch depth intervals from surface to the top of the varved clay. The CPT rods were removed from the subsurface at the completion of ROST<sup>TM</sup> data collection. The open hole was then immediately tremie grouted using a cement/bentonite mixture. Fugro dedicated a man to operating a portable mixer and pump for the tremie grouting of the ROST<sup>TM</sup> borings. The pump, mixer and tremie pipe were loaded onto a trailer and towed behind a pickup truck. The pickup truck followed the CPT such that it could occupy a ROST<sup>TM</sup> boring immediately after it was vacated. Tremie grouting was performed under the supervision of a licensed New Jersey driller to insure that it was properly completed.

Several drops of DNAPL were obtained from locations MW-3L, MW-8L, MW-12L, MW-13L and MW-14L. These samples were placed on the ROST<sup>TM</sup> for purposes of calibrating the results to the Site conditions.

Four additional HSA soil borings were performed at ROST<sup>TM</sup> locations R-8, R-24, R-26 and R-30. The purpose of the additional borings was to confirm the presence or absence of DNAPL as indicated by the ROST<sup>TM</sup> data. The borings were advanced to the depth at which DNAPL was observed with the ROST<sup>TM</sup>. A split spoon sample was then obtained at that depth and visually examined for the presence of NAPL. The sample was then placed in a jar and a headspace analysis was performed using a PID to qualitatively determine the presence or absence of free phase and dissolved compounds. NAPL was observed at locations R-8, R-24 and R-26. DNAPL was not observed at location R-30. The boreholes were then immediately tremie grouted upon completion using a cement bentonite grout.

#### 2.7 DNAPL RECOVERY EVALUATION

A DNAPL recovery evaluation was performed over a two-week period (February 3 to February 16, 1999) to determine the feasibility of DNAPL recovery from the sand unit using existing monitoring wells. The four groundwater monitoring wells containing a measurable DNAPL layer (MW-3L, MW-8L, MW-12L and MW-13L) were used for this evaluation.

Prior to the initial DNAPL removal event, the apparent DNAPL thickness in each well was measured using an oil/water interface probe. The DNAPL was removed from these wells using a peristaltic pump and the volume of DNAPL removed was measured in a calibrated bucket. Following evacuation of the DNAPL, the DNAPL recovery in the wells was monitored. Once the DNAPL thickness had returned to a level at or near its pre-test thickness, the DNAPL was again removed. This process was repeated several times during the two-week testing period.

#### 2.8 SITE SURVEY

A Site survey was performed by Neglia and Associates, a New Jersey licensed professional surveyor. All ROST<sup>TM</sup> sounding and soil boring locations were surveyed to determine their horizontal and ground surface elevation. The newly installed groundwater monitoring wells were surveyed to determine the top-of-casing elevation, as well as, horizontal coordinates and ground surface elevation. Survey data is included in Appendix C. The surveyor also obtained the location of several building corners for the purpose of adjusting the base map that was believed to have been created from a non-stereo rectified aerial photograph. The location of the top of the lagoon berm and the elevation of the lagoon water surface were also obtained during this surveying effort.

All survey data were obtained in New Jersey State Plane Coordinates and in units of feet. The data were referenced to the horizontal North American Datum of 1983 (NAD83) and the North American Vertical Datum of 1988 (NAVD88).

#### 3.0 SUPPLEMENTAL REMEDIAL INVESTIGATION RESULTS

The findings of the SRI field activities are described in this section. Soil data are compared to both the New Jersey Non-Residential Direct Contact Soil Cleanup Criteria (NRDCSCC), the Residential Direct Contact Soil Cleanup Criteria (RDCSCC), and the Impact to Groundwater Soil Cleanup Criteria (IGWSCC). Groundwater data are compared the New Jersey Class II-A Groundwater Quality Criteria (GWQC). One copy of the analytical data packages received from the laboratory has been forwarded to the NJDEP Case Manager under separate cover, as allowed for under N.J.A.C. 7:26E-3,13(c).

#### 3.1 TRANSFORMER AREA SOIL SAMPLING AND ANALYSIS

The results of the PCB analyses are summarized in Table 3-1. The primary PCB analyses was performed using EPA Method 8081. Analyses by EPA Method 8082 was used for confirmational purposes. Comparison of the data presented on Table 3-1 indicate agreement of the concentrations measured by the two methods.

The analytical results indicate that the PCB concentration measured in the concrete chip sample exceeds the NRDCSCC. A total PCB concentration of 6.800 mg/kg was measured in this sample. This was concentration was entirely attributable to the presence of PCB-1260. Total PCB concentrations in the three surface soil samples collected in this area were less than the RDCSCC. Therefore, no further investigation of the presence of PCB in the transformer area is necessary.

#### 3.2 MW-3L AREA SOIL SAMPLING AND ANALYSIS

The soil samples from the soil borings completed in the MW-3L area in January 1999 were analyzed for SVOCs. The results of these analyses are summarized in Table 3-2. The primary objective of the sampling and analysis program was to provide data to support an evaluation of the source of constituents previously detected in groundwater in monitoring well MW-3L. Also, the results of these analyses provide useful information for defining surface and subsurface soil conditions for the western portion of the SCCC Site. The following points summarize the soil analytical results:

• Concentrations of all SVOCs in the surface soil samples from locations SB-16. MW-16L and MW-17L are less than the RDCSCC. Benzo(a)anthracene. benzo(b)fluoranthene,benzo(k)fluoranthene and bis(2-ethylhexyl)phthalate(BEHP). were detected at concentrations greater than the RDCSCC in the surface soil sample collected from SB-15. However, the concentrations of benzo(a)anthracene. benzo(b)fluoranthene, benzo(k)fluoranthene are less than the NRDCSCC. The

BEHP concentration is slightly greater than the NRDCSCC. Only one location (SB-15), at the 0-2 foot interval, exceeds the New Jersey IGWSCC. The only constituents to exceed the IGWSCC in this sample are BEHP and di-n-octyl phthalate.

- Concentrations of all SVOCs in the soil samples collected from directly above the meadow mat at locations SB-15, SB-16, MW-16L and MW-17L are less than the RDCSCC. The results of these analyses indicate that a constituent source within the fill materials does not exist at these locations. This also indicates that migration of SVOCs observed in the surface soils at location SB-15 has not occurred.
- Concentrations of all SVOCs in the soil samples collected from directly above the varved clay at locations SB-15 and SB-16 are less than the RDCSCC. The results of these analyses indicate that these borings are not located along a constituent migration pathway within the sand unit.
- Based on the soil analytical results, further delineation of the extent of constituents in surface soils in the western portion of the SCCC Site is not necessary particularly since the Site is zoned industrial.

#### 3.3 GROUNDWATER SAMPLING AND ANALYSIS

The groundwater samples collected from new monitoring wells MW-16L and MW-17L were analyzed for SVOCs. The results of these analyses are summarized in Table 3-3. The objective of the groundwater sampling and analysis program was to provide additional data for evaluation of the extent of constituents in groundwater in the sand unit in the general vicinity of monitoring well MW-03. The following points summarize the groundwater analytical results:

- Concentrations of all SVOCs, with the exception of 1,4-dichlorobenzene, were less than the GWQC in monitoring well MW-16L. The concentration of 1,4-dichlorobenzene in this sample was 540 ug/l.
- Concentrations of the three dichlorobenzene isomers and 1,2,4-trichlorobenzene were
  detected at concentrations greater than the GWQC Class IIA in monitoring well MW17L.

Monitoring wells MW-16L and MW-17L are located along the western and southern boundaries, respectively. Additional groundwater characterization within the western portion of the SCCC Site is not warranted.



## 3.4 GROUNDWATER FLOW

The depth to groundwater was measured in site monitoring wells prior to the groundwater sampling event. These data were used to calculate potentiometric elevations at each monitoring well location. The data are summarized on Table 3-4. The potentiometric elevation data for groundwater in the fill unit and sand unit were contoured for interpretation of groundwater flow directions. The potentiometric surface elevation contour maps for the fill unit and sand unit are included as Figures 3-1 and 3-2, respectively.

Groundwater flow patterns observed in the fill unit are consistent with those observed in the past. A potentiometric mound is present in the vicinity of the lagoon. Shallow groundwater flows radially outward from this mound. In areas beyond the influence of the potentiometric mound, groundwater in the fill appears to flow in a southerly direction, approximately parallel to the Hackensack River, under the influence of a relatively flat horizontal hydraulic gradient.

Groundwater flow patterns observed in the sand unit were also consistent with those previously observed. A potentiometric mound is present in the area to the northwest of the lagoon. Groundwater flow from this mound is to the east and the south. The horizontal hydraulic gradients in the vicinity of the lagoon appear to be greater than those indicated in other areas of the property. This depiction may be due to tidal influences in the area of monitoring wells MW-8L and MW-9L. In areas beyond the influence of the potentiometric mound, groundwater in the sand appears to flow in a southeasterly direction, sub-parallel to the Hackensack River, under the influence of a relatively flat horizontal hydraulic gradient. The difference in potentiometric elevations across the Site in areas not affected by the mound and tidal influences is only 0.5 feet.

#### 3.5 ROST™ SURVEY RESULTS

The evaluation of the distribution of DNAPL in the fill and sand units at the Site was further evaluated by performing ROST<sup>TM</sup> soundings at 31 locations. At each location, the soundings were advanced to an elevation below the contact to between the sand unit and the underlying varved clay to allow for an evaluation of DNAPL presence at the base of the sand unit.

The inferred presence of DNAPL in the subsurface was made based on elevated fluorescence intensity and a match of a characteristic fluorescence multi-length waveform. At the SCCC Site, two characteristic fluorescence waveforms were identified that, when detected in conjunction with elevated fluorescence intensities, are interpreted to be indicative of DNAPL presence. These characteristic fluorescence signatures are shown on Figure 3-3.

One DNAPL signature is characterized by the maximum fluorescence intensity emitted at a wavelength of 440 nm. This DNAPL signature was, for the most part, more prevalent in the ROST<sup>TM</sup> soundings completed in the vicinity of Buildings 2, 3, and 4. Based on the groundwater analytical data for samples collected in this area, this DNAPL is believed to be comprised primarily of the dichlorobenzene isomers.

The other DNAPL signature is characterized by maximum fluorescence intensity emitted at a wavelength of 390 nm. This DNAPL signature was, for the most part, more prevalent in the ROST<sup>TM</sup> soundings completed in the eastern portion of the Site in the vicinity of the lagoon. Based on the groundwater analytical data for samples collected in this area, this DNAPL is believed to be comprised primarily of the dichlorobenzene isomers, naphthalene and trichlorobenzene isomers.

#### 3.5.1 DNAPL Distribution in the Fill Unit

Figure 3-4 shows the locations where the ROST<sup>TM</sup> data indicates the presence of DNAPL in the shallow unit. The ROST<sup>TM</sup> indicates the presence of DNAPL at three sounding locations (R-3, R-4, and R-7) in the vicinity of Buildings 2, 3 and 4. The possible presence of DNAPL at these locations is inferred based on elevated fluorescence intensity and a multi-wavelength signature showing the greatest fluorescence emitted at a wavelength of 440 nm. At these three locations, the DNAPL appears as a very thin layer (less than 1 ft.) perched on top of the meadow mat. As indicated on Figure 3-4, the distribution of DNAPL in the fill unit in this area is limited in extent as the ROST<sup>TM</sup> data from surrounding borings in this area do not indicate the presence of DNAPL.

To confirm the limited extent of DNAPL in the fill unit in the vicinity of Buildings 2, 3, and 4, information from previous investigations was reviewed. This information includes visual observations and analytical data from soil borings and monitoring wells completed during the Weston RI. Information acquired from Weston borings SB-1 through SB-4 and monitoring well MW-15U do not suggest the presence of DNAPL at these locations. Based on the ROST<sup>TM</sup> data and information from the Weston RI, DNAPL in the fill unit in the vicinity of Buildings 2, 3 and 4 has not migrated laterally.

The ROST<sup>TM</sup> data indicate the possible presence of DNAPL in the fill unit at three locations (R-12, R-15 and R-16) in immediately adjacent to the lagoon. At these locations, the presence of DNAPL is indicated by a different fluorescence signature than the DNAPL observed in the vicinity of Building 4, with the greater fluorescence emitted at a wavelength of 390 nm. The DNAPL at locations R-12 and R-15 appears as a thin layer perched on top of the meadow mat. As indicated on Figure 3-4, the distribution of DNAPL in the fill unit in this area is limited in extent as the ROST<sup>TM</sup> data from most soundings completed in the vicinity of the lagoon do not indicate the

presence of DNAPL. These data indicate that lateral migration of DNAPL within the fill unit is minimal.

Boring log information and analytical data from the Weston RI and the ERM FRI verify the limited extent and lateral movement of DNAPL in the fill unit in the vicinity of the lagoon. However, visual observations made by Weston and ERM suggest the presence of a localized source in the fill unit in the vicinity of monitoring well MW-7L and SB-7 located in the vicinity of the northern property boundary.

#### 3.5.2 DNAPL Distribution in the Sand Unit

Figure 3-5 shows the locations where the ROST<sup>TM</sup> data indicates the presence of DNAPL in the sand unit. As indicated on Figure 3-5, DNAPL is more widely distributed in the sand unit than in the overlying fill.

In the vicinity of Buildings 2, 3, and 4, the DNAPL exhibits the same fluorescence characteristics as the DNAPL identified in the shallow zone in this area. The DNAPL appears to be distributed as a very thin layer perched directly on top of the varved clay. The extent of DNAPL in the Building 2, 3 and 4 area is delineated on-Site to the north by ROST<sup>TM</sup> soundings R-1, R-28 and R-29, to the east by R-30 and R-31 and to the west by MW-16L.

The elevated fluorescence intensity readings in the sand unit in the lagoon area typically correspond to a signature showing the greater fluorescence intensity being emitted at a wavelength of 390 nm. The ROST<sup>TM</sup> data indicate the extent of DNAPL to the west of the lagoon area has been delineated by soundings R-10, R-30 and R-31. The ROST data indicate that DNAPL is present in the sand unit at the northern property boundary from the northeastern corner of the property to the west to sounding R-24. DNAPL was not detected at sounding R-32 which was completed along the northern property to the west of R-24.

The ROST<sup>TM</sup> data indicate the presence of DNAPL in the sand unit in the area between the lagoon and the Hackensack River. The inferred DNAPL thicknesses in this area based on the ROST<sup>TM</sup> data from soundings R-14, R-15, R-16A and R-21, are somewhat greater than those observed in other portions of the Site. DNAPL is also inferred to be present in the area to the south of the lagoon.

## 3.5.3 DNAPL Migration Pathways

The CPT data and drilling results from the January 1999 mobilization were used to create structure maps of the top of the meadow mat and the top of the varved clay (Figures 3-6 and 3-7). The

structure maps were created through the use of the geostatistical software OASIS Montage. The structure contour map of the meadow map surface indicates that a topographic high exists at the northeastern extreme of the Site. The meadow mat surface appears to gently dip to the southwest towards the former lagoon area. Based on the structure of the meadow map surface in the lagoon area, migration of DNAPL from the lagoon to locations R-12, R-15 and R-16 is possible. Soundings R-12 and R-15 appear to be located within a slight depression within the upper surface of the meadow mat. The low relief on the top of the meadow mat surface in the vicinity of the former lagoon area is consistent with the limited extent of lateral DNAPL migration within this unit.

In the Building 4 area, soundings R-3 and R-4, where the presence of DNAPL was inferred in the fill unit, are located at a local topographic high in the meadow mat surface. This information suggests that these soundings were completed in close proximity to a DNAPL source. The dip of the meadow mat surface to the south in this area is consistent with the inferred presence of DNAPL in the fill unit at location R-7.

The structure contour map for the top of the varved clay unit indicates that this surface slopes to the northeast and northwest in the vicinity of the lagoon area. This is consistent with the distribution of DNAPL in this area. In the vicinity of Buildings 2, 3, and 4, the varved clay unit dips from a local high at the inferred source area near sounding R-3 to the southwest towards well MW-3L and MW-17L. The slope of this surface is consistent with the presence of DNAPL in well MW-3L.

The vertical distribution of DNAPL is depicted on geologic cross-sections A-A'. B-B' and C-C'. The plan location of the cross-sections are shown as Figure 3-8 and the cross-sections are presented as Figures 3-9A through 3-9C. The geologic cross-sections are based on the CPT data and the drilling data acquired during the January 1999 mobilization. The results of the ROST<sup>TM</sup> logs are overlain onto the cross sections to depict the relationship between DNAPL presence and Site stratigraphy.

#### 3.6 DNAPL RECOVERY EVALUATION

The results of the DNAPL recovery evaluation indicate that removal of DNAPL from the sand unit using existing wells (MW-3L, MW-8L, MW-12L and MW-13L) is feasible. The total volume of DNAPL recovered from individual wells over the two week test period ranged from 7.2 gallons from monitoring well MW-8L to 12.55 gallons from monitoring well MW-3L. Thus, DNAPL recovery rates range from approximately 0.5 gallons to 1 gallon per day. Data recorded during the DNAPL recovery data is included in Appendix D:

#### 4.0 CONCLUSIONS AND RECOMMENDATIONS

The objectives of the SRI were met through the implementation of the field activities described in this report. The surface soil sampling conducted in the transformer area completed the delineation of PCBs and confirmed the limited extent of impacted media in this area.

The soil sampling and analysis program completed the characterization of soil conditions in the western area. These data indicate that a source area does not exist in the extreme western portion of the SCCC Site. These data also indicate that potential impact to surface soils in this portion of the Site is limited to the vicinity of SB-15.

The on-Site characterization of groundwater in the western portion of the facility was completed through installation and sampling of monitoring wells MW-16L and MW-17L. Elevated levels of dichlorobenzene isomers were detected in monitoring well MW-17L. These constituents are likely attributable to the DNAPL present in the sand unit at location MW-3L.

The CPT/ROST<sup>TM</sup> survey further defined DNAPL source areas, potential migration pathways and distribution in the fill and sand units. The ROST<sup>TM</sup> data indicate that lateral movement of DNAPL within the fill unit is limited. These data indicate that the distribution of DNAPL in this zone remains in close proximity to the suspected source areas (the lagoon and Buildings 2 and/or 4 areas). In most instances, the DNAPL identified in the shallow unit exists as a thin layer perched directly above the meadow mat. No further investigation regarding the lateral extent of DNAPL in the fill unit is recommended at this Site.

The ROST<sup>TM</sup> data indicate that the extent of DNAPL in the sand unit is more widespread than in the overlying fill unit. The movement of DNAPL in the sand unit appears to be controlled somewhat by the slope of the top of the underlying varved clay unit. In the lagoon area, DNAPL appears to have migrated along the top of clay to the northeast and the northwest. DNAPL was also observed to be present south of the lagoon in areas where the varved clay is at a topographic high. Significant DNAPL migration also appears to have occurred from the Buildings 2 and/or 4 areas to the southwest to the location of monitoring well MW-3L. The top of the varved clay slopes downward, from the Buildings 2 and/or 4 area, towards MW-3L and MW-17L.

The results of the DNAPL recovery evaluation demonstrate that DNAPL can be removed from the sand unit cost-effectively and efficiently using the existing monitoring wells.

With the data acquired through implementation of the SRI, sufficient information exists to prepare a Remedial Action Selection Report (RASR) for the Site. In accordance with the ACO, the RASR



will be submitted to NJDEP within 90 days of receipt of written approval of this SRI Report from NJDEP. In conjunction with preparation of the RASR, limited additional field data may be acquired to support remedy selection and/or design. Also, SCCC will initiate the removal of DNAPL from existing monitoring wells MW-3L, MW-8L, MW-12L and MW-13L through the RASR and RAWP preparation.

A summary of the SRI conclusions for this Site is as follows:

- 1. The objectives of the SRI were met through the implementation of the field activities described in this report.
- 2. The former production well was successfully abandoned by a licensed New Jersey drilling subcontractor.
- 3. A source area does not exist in the extreme western area of the Site.
- 4. On-Site characterization of the groundwater at the western portion of the Site was completed through the installation and sampling of monitoring wells MW-16L and MW-17L.
- 5. The CPT/ROST<sup>TM</sup> survey indicated that the suspected DNAPL source areas for this Site are the lagoon, and Building 2 and/or Building 4.
- 6. CPT/ROST<sup>TM</sup> survey data indicated that the lateral extent of DNAPL in the fill unit is limited and not extensive.
- 7. CPT/ROST<sup>TM</sup> survey indicated that where DNAPL is present in the fill unit it exists as a thin perched layer located immediately above the meadow mat.
- 8. CPT/ROST<sup>™</sup> survey indicated that the presence or absence of DNAPL in the sand layer is partly controlled by the slope of the surface of the varved clay unit relative to the suspected source areas.
- 9. The ROST<sup>TM</sup> results for the area south of the lagoon are not consistent with previous investigations, analytical results, or the topographic surface of the varved clay.
- 10. This SRI indicated that modest DNAPL recovery within the sand layer can be accomplished through the existing groundwater monitoring well network.

11. Sufficient data exists to prepare a RASR for this Site.

A summary of the SRI recommendations for this Site is as follows:

- 1. No further sampling is required at the former PCB transformer area as the limited extent of impacted media at this location was sufficiently delineated as part of this SRI.
- 2. No further sampling is required in the western portion of the Site as our most recent analytical data indicates that a source area does not exist within this area.
- 3. No further investigation of the lateral extent of DNAPL in the fill unit is recommended as our most recent CPT/ROST<sup>TM</sup> has sufficiently defined its limited extent.
- 4. Low volume DNAPL recovery using the existing groundwater monitoring wells should be considered as an efficient and cost effective method for removing DNAPL from the sand layer.
- 5. Low volume DNAPL recovery using the existing groundwater monitoring wells should continue through the RASR and the RAWP preparation.

**TABLES** 

## TABLE 2-1A

# FORMER PRODUCTION WELL GROUNDWATER ANALYTICAL RESULTS FOR VOLATILE ORGANIC COMPOUNDS STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY

	Parameter Company		3 (5%) 17-	
SITE:		SC-PW	SC-PW	SC-PW
DATE:		10/8/98	10/8/98	10/8/98
DEPTH (ft):		88	228	360
CONSTITUENT:	UNITS			
1,1,1-Trichloroethane	ug/ <b>L</b>	0.5 U	0.5 U	0.5 U
1,1,2,2-Tetrachloroethane	ug/ <b>L</b>	0.9 U	0.9 U	0.9 U
1,1,2-Trichloroethane	ug/L	1.9 U	1.9 U	1.9 U
1,1-Dichloroethane	ug/L	0.6 U	0.6 U	0.6 U
1,1-Dichloroethylene	ug/L	1.1 U	1.1 U	1.1 U
1,2-Dichloroethane	ug/L	1.3 U	1.3 U	1.3 U
1,2-Dichloropropane	ug/L	0.5 ป	0.5 U	0.5 Ù
2-Hexanone	ug/ <b>L</b>	3 U	3 U	3 U
Acetone .	ug/L	_ 38 B	10 B	6.2 JB
Benzene	ug/L	0.7 U	0.7 U	0.7 U
Bromodichloromethane	ug/L	0.8 U	0.8 U	0.8 U
Bromoform	ug/l_	2.5 U	2.5 U	2.5 U
Carbon disulfide	ug/L	2.4 U	2.4 U	2.4 U
Carbon tetrachloride	ug/L	1 U	1 U	1 U
Chlorobenzene	ug/L	1.1 U	1.1 U	1.1 U
Chloroethane	ug/L	1.3 U	1.3 U	1.3 U
Chloroform	ug/L	0.8 U	0.8 U	0.8 U
cis-1,2-Dichloroethylene	ug/ <b>L</b>	0.8 ∪	0.8 U	0.8 U
cis-1,3-Dichloropropene	ug/ <b>L</b>	0.7 U	0.7 U	0.7 U
Dibromochloromethane	ug/ <b>L</b>	1.7 U	1.7 U	1.7 U
Ethylbenzene	ug/ <b>L</b>	2.2 U	2.2 U	2.2 U
Methyl bromide	ug/L	2.3 U	2.3 U	2.3 U
Methyl chloride	ug/L	7.7 U	7.7 U	7.7 U
Methyl ethyl ketone	ug/ <b>L</b>	2.5 JB	2.4 JB	2.5 U
Methyl isobutyl ketone (MIBK)	ug/L	2.7 U	2.7 U	2.7 U
Methylene chloride	ug/L	1.2 U	1.2 U	1.2 U
Styren <b>e</b>	ug/L	1.8 U	1.8 U	1.8 U
Tetrachloroethylene	ug/L	0.8 U	0.8 U	0.8 U
Toluen <b>e</b>	ug/L	0.8 U	0.8 U	0.8 U
trans-1,2-Dichloroethene	ug/L	0.7 U	0.7 U	0.7 U
trans-1,3-Dichloropropene	ug/ <b>L</b>	1.5 U	1.5 U	1.5 U
Trichloroethylene :	ug/L	0.6 ป	0.6 U	0.6 U
Vinyl chloride	ug/L	1.2 U	1.2 U	1.2 U
Xylen <b>es</b>	ug/L	1.3 U	1.3 U	1.3 U
				· ·

U - indicates compound not detected at listed detection limit.

B - indicates compound detected in associated blank.

J - indicates estimated concentration less than the method detection limit.

## TABLE 2-1B

## FORMER PRODUCTION WELL GROUNDWATER ANALYTICAL RESULTS FOR SEMI-VOLATILE ORGANIC COMPOUNDS STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY

		SC-PW	CO DW	SC-PW
SITE:			SC-PW	10/8/9 <b>8</b>
DATE:		10/8/9 <b>8</b> 8 <b>8</b>	10/8/9 <b>8</b> 22 <b>8</b>	360
DEPTH (ft):			220	300
CONSTITUENT:	UNITS			
SEMI-VOLATILES				
1,2,4-Trichlorobenzene	ug/L	0.44 U	0.44 U	0.44 U
2,4,5-Trichlorophenol	ug/L	0.42 U	0.42 U	0.42 U
2,4,6-Trichlorophenol	ug/L	0.58 U	0.58 ∪	0.58 U
2,4-Dichlorophenol	ug/L	0.54 U	0.54 U	0.54 U
2,4-Dimethylphenol	ug/L	0.54 U	0.54 U	0.54 U
2,4-Dinitrophenol	ug/L	12 U	12 U	12 U
2.4-Dinitrotoluene	ug/L	0.28 U	0.28 U	0.28 U
2,6-Dinitrotoluene	ug/L	0.39 U	0.39 U	0.39 ป
2-Chloronaphthalene	ug/L	0.42 U	0.42 U	0.42 U
2-Chlorophenol	ug/L	0.85 U	0.85 U	0.85 U
2-Methylnaphthalene	ug/L	0.34 U	0.34 U	0.34 U
3,3-Dichlorobenzidine	ug/L	3.6 U	3.6 U	3.6 U
4,6-Dinitro-o-cresol	ug/L	0.45 U	0.45 U	0.45 U
4-Bromophenyl phenyl ether	ug/L	0.6 U	0.6 U	0.6 U
4-Chlorophenyl phenyl ether	ug/L	0.4 U	0.4 U	0.4 U
Acenaphthene	ug/L	0.48 U	0.48 U	0.48 U
Acenaphthylen <b>e</b>	ug/ <b>L</b>	0.38 U	0.38 U	0.38 U
Anthracen <b>e</b>	ug/L	0.41 U	0.41 U	0.41 <b>U</b>
Benzo(a)anthracene	ug/L	2.2	2.4	3
Benzo(a)pyrene	ug/L	3.8	4.4	5.6
Benzo(b)fluoranthene	ug/L	5.2	4.9	6.9
Benzo(ghi)perylene	ug/L	3.5	4.2	5.3
Benzo(k)fluoranthene	ug/ <b>L</b>	<b>1.3</b>	1.9	2.2
Bis(2-chloro-1-methylethyl) ether	ug/L	0.68 U	0.68 U	0.68 U
Bis(2-chloroethoxy)methane	ug/L	0.39 U	0.39 U	0.39 U
Bis(2-chloroethyl)ether	ug/ <b>L</b>	0.28 U	0.28 U	0.28 U
Bis(2-ethylhexyl)phthalate (BEHP)	ug/ <b>L</b>	1.9	5.4	5.8
Butyl benzyl phthalate	ug/ <b>L</b>	0.54 U	0.54 U	0.54 U
Carbazole	ug/L	0.41 U	0.41 U	0.41 U
Carbolic acid	ug/L	0.45 ป	0.45 U	0.45 U
Chrysen <b>e</b>	ug/L	2.6	3	4.1
Dibenzo(a,h)anthracene	ug/ <b>L</b>	0.47 U	0.47 U	1
Dibenzofuran	ug/L	0.42 U	0.42 U	0.42 U
Diethyl phthalate	ug/L	0.45 U	0.45 U	0.45 U
Dimethyl phthalate	ug/L	0.43 U	0.43 U	0.43 U
Di-n-butyl phthalate	ug/ <b>L</b>	0.85 U	0.85 U	0.85 ป
22031		000	<u> </u>	<u> </u>

Page 1 of 2 0 3 6

TABLE 2-1B

# FORMER PRODUCTION WELL GROUNDWATER ANALYTICAL RESULTS FOR SEMI-VOLATILE ORGANIC COMPOUNDS STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY

SITE: DATE: DEPTH (ft):		SC-PW 10/8/98 88	SC-PW 10/8/98 228	SC-PW 10/8/98 360
CONSTITUENT:	UNITS			
SEMI-VOLATILES (Continued)				The second secon
Di-n-octyl phthalate	ug/L	0.43 U	0.43 Ü	0.43 U
Fluoranthene	ug/L	2.8	3	3.7
Fluorene	ug/L	0.33 U	0.3 <b>3</b> U	0.33 U
Hexachlorobenzene	ug/L	0.39 U	0.39 U	0.39 U
Hexachlorobutadiene	ug/L	0.54 U	0.54 U	0.54 U
Hexachlorocyclopentadiene	ug/L	1.5 U	1.5 U	1.5 U
Hexachloroethane	ug/L	0.5 U	0.5 U	0.5 U
Indeno(1,2,3-cd)pyrene	ug/L	2.8	3.4	4.3
Isophorone	ug/L	0.3 U	0.3 U	0.3 U
m-Dichlorobenzene	ug/L	0.5 U	0.5 U	0.5 U
m-Nitroanili <b>ne</b>	ug/L	1.4 U	1.4 U	1.4 U
Naphthale <b>ne</b>	ug/L	0.38 U	0.38 U	0.38 U
Nitrobenzen <b>e</b>	ug/L	0.36 U	0.36 U	0.36 U
N-Nitrosodiphenylamine	ug/L	0.42 U	0.42 U	0.42 U
N-Nitrosodipropylamin <b>e</b>	ug/L	0.39 U	0.39 U	0.39 U
o-Cresol	ug/L	0.71 U	0.71 U	0.71 U
o-Dichlorobenzen <b>e</b>	ug/L	0.61 <b>U</b>	0.61 U	0.61 U
o-Nitroanili <b>ne</b>	ug/L	0.43 U	0.43 U	0.43 U
o-Nitrophe <b>nol</b>	ug/L	0.58 U	0.5 <b>8 U</b>	0.58 U
p-Chloroaniline	ug <b>/L</b>	0.25 U	0.25 ປ	0.25 ປ
p-Chloro-m-cresol	ug/L	0.57 <b>U</b>	0.57 U	0.57 U
p-Cresol	ug/L	0.24 <b>U</b>	0.24 U	0.24 U
p-Dichlorobenzene	ug/ <b>L</b>	0.59 U	0.59 U	0.59 U
Pentachlorophenol	ug/ <b>L</b>	0.48 <b>U</b>	0.48 U	0.48 U
Phenanthrene	ug/L	1.3	1.3	1.6
p-Nitroaniline	ug/ <b>L</b>	0.35 <b>U</b>	0.35 U	0.35 U
p-Nitrophen <b>ol</b>	ug/L	4.9 U	4.9 U	4.9 U
Pyrene Pyrene	ug/L	2.6	3	0.43 U

U - indicates compound not detected at listéd detection limit.

#### TABLE 2-1C

## FORMER PRODUCTION WELL GROUNDWATER ANALYTICAL RESULTS FOR METALS STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY

SITE: DATE: DEPTH (ft):		SC-PW 10/8/98 88	SC-PW 10/8/98 228	SC-PW 10/8/98 360
CONSTITUENT:	UNITS			
Alumin <b>um</b>	ug/L	798	527	533
Antimony	ug/L	2.31 B	1.7 B	1.65 B
Arsenic	ug/L	2.12 B	1.525 U	1.8 B
Bariu <b>m</b>	ug/L	35.3	67.7	213
Berylliu <b>m</b>	ug/L	0.19 B	0.27 B	0.44 B
Cadmium	ug/L	0.215 B	0.125 B	0.125 B
Calcium	ug/L	28300	30 <b>500</b>	105000
Chromium	ug/L	9 <b>89</b>	819	890
Cobalt	ug/L	3.17 B	2.3 B	2.04 B
Copper	ug/L	15.9	33.7	30.6
Iron	ug/L	3590	2630	2070
Lead	ug/L	16.4	12.9	11.6
Magnesiu <b>m</b>	ug/L	10400	14800	77200
Manganese	ug/L	55.2	33	48.1
Mercury	ug/L	0.2 U	0.2 U	0.2 U
Nickel	ug/ <b>L</b>	86. <b>8</b>	65. <b>8</b>	59. <b>3</b>
Potassiu <b>m</b>	ug/L	1700 <b>0</b>	1490 <b>0</b>	3770 <b>0</b>
Selenium .	ug/ <b>L</b>	2.63	2.02 B	3.98
Silver	ug/L	0.294 U	0.294 U	0.294 U
Sodium	ug/L	17800 <b>0</b>	23200 <b>0</b>	6950 <b>00</b>
Thallium :	ug/ <b>L</b>	2.66 B	2.36 B	2.75 B
Vanadiu <b>m</b>	ug/L	167	163	146
Zinc	ug/L	5 <b>2.8</b>	43.5	38.3

U - indicates compound not detected at listed detection limit.

B - indicates reported concentration is greater than the instrument detection limit but less than the contract required detection limit.

TABLE 3-1

### SOIL ANALYTICAL RESULTS FOR PCB's STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY

CONSTITUENT:	Non-Residential Direct Contact	Direct Contact	SITE: DESCRIPTION: DATE: DEPTH (ft):	TA-CC01 CONCRETE CHIP 10/9/98 0" - 1"	TA-SS01 SURFACE SOIL 10/9/98 0" - 6"	TA-SS02 SURFACE SOIL 10/9/98 0" - 6"	TA-SS03 SURFACE SOIL 10/9/98 0" - 6"
(Units in mg/kg)	Criteria	Criteria	ANALYTICAL METHOD:	8081 8082	8081 8082	8081 8082	8081 8082
PCB-1016				< 340 < 28	< 0.034 < 0.055	< 0.034 < 0.055	< 0.036 < 0.055
PCB-1221				< 340 < 6	< 0.034 < 0.011	< 0.034 < 0.011	< 0.036 < 0.011
PCB-1232				< 340 < 28	< 0.034 < 0.055	< 0.034 < 0.055	< 0.036 < 0.055
PCB-1242				< 340 < 28	< 0.034 < 0.055	< 0.034 < 0.055	< 0.036 < 0.055
PCB-1248				< 340 < 31	< 0.034 < 0.061	< 0.034 < 0.061	< 0.036 < 0.061
PCB-1254				< 340 < 22	< 0.034 < 0.044	< 0.034 < 0.044	< 0.036 < 0.044
PCB-1260				6800 9300	0.15 0.12	0.16 0.29	0.022 J < 0.076
PCB's (Total)	2	0.49		[6800] [9300]	0.15 0.12	0.16 0.29	0.022 J ND

<sup>[ ] -</sup> Indicates sample concentration greater than New Jersey Non-Residential Soil Cleanup Criteria.

J - Estimated concentration less than the method detection limit.

ND - Not Detected.

SOIL ANALY SEMI - VOLATILE ( STANDARD CHLORII KEARNY,

	NEW JERSEY	NEW TOTAL TO	NEW CONTRACTOR	Cally Saleson Stewart	worden inthing herbored	THE RESERVE OF THE SECOND
	INPACT TO	JERSEY	10/30	SITE	SCMW-18L	SC-MW-16L
	The state of the s	NON RESIDENTIAL	RESIDENTIAL	DATE:	ากวงจะ วารา	1/13/09
		DIRECT CONTACT	DIFFECT CONTACT	DEPTH (m)	0.2	8 12
· · · · · · · · · · · · · · · · · · ·		والمراكز المحودث والمحال والمراكز والمراكز والمراجع والمراجع والمراجع والمراجع	SOIL CLEAMUP CRITERIA		ANY STATE OF THE STATE OF	
Inits in ug/kg)	CRITERIA SISSASSIA	SOIL CLEANUP CRITERIA	SOIL CLEAMUP CHOTERIA (574)	The many was a season	and a production of the acceptance of the a	1. 10, 10 1
he <b>nol</b>	50000	10000000	10000000		< 380	< 440
is(2-chloroethyl)ether	10000	3000	660		< 120	< 140
Chlorophen <b>ci</b>	10000	5200000	2800 <b>00</b>		< 380	< 440
3-Dichlorobenzene	100000	10000000	5100000		75 J	540
4-Dichlorobenzene	100000	10000000	5700 <b>00</b>		< 120	2000
2-Olchlorobenzene	50000	10000000	51000 <b>00</b>		76 J	190
	2000	1000000			< 380	< 440
Methylphenol		10000000	28000 <b>00</b>			
ropane, 2,7-oxybis[1-chloro-	1				< 120	< 140
Melhylphenal -	ì	10000000	28000 <b>00</b>		< 380.	< 440
-Nirozodipropylamine	10000	<b>e60</b>	660		< 120	< 140 ·
exachloroeth <b>ane</b>	100000	100000	6000	•	< 120	< 140
irobenze <b>ne</b>	50000	50000	28000	· ·	< 120	< 140
ophorone	50000	10000000	1100000		< 120	< 140
Nitrophenel					< 380	< 440
. ,	10000	10000000	1100000	. *	< 360	< 440
4_Dimethylphenel	, 1000	,,,,,,,,,,,	110000		< 120	< 140
ls(2-chloroethoxy)methana				1.		
4-Dichlorophenel	10000	3100000	170000		< 380	< 440
2,4-Trichlorobenzena	100000	1200000	68 <b>000</b>		< 120	< 140
aphinal <del>ens</del>	100000	420000 <b>0</b>	2300 <b>00</b>		< 120	< 140
Chiorosnifine	a a garage 📢	4200000	230 <b>000</b>	• .	< 120	< 140
exachlorobumdiene	100000	21000	1000		< 120	< 140
Chloro-3-methylphenol	100000	10000000	10000000		< 120	< 140
Methylnaphthalena					< 120	< 140
exachlorocyclopentadiene	100000	7300000	400 <b>000</b>		< 120	< 140
4.6-Trichiorophen <b>ci</b>	10000	270000	620 <b>00</b>		< 380	< 440
4.5-Trichlorophenol	50000	10000000	5600000		< 2000	< 2300
	2000	10000000	200000			
Chloronaphthalene	• 1				< 120	< 140
Nitroaniline	1 8 2 2 2 2				< 2000	< 2300
lmeinyiphthelate	50000	. 10000000	10000000	, .	< 120	< 140
cenaphthylene					<b>62                                   </b>	< 140
6-Dinitrotoluene		the second of the second			< 120	< 140
Nitrophiling	'				< 2000	< 2300
conaphthene	100000	10000000	3400000		< 120	< 140
4-Dinitrophenol	10000	2100000	110000		< 2000	< 2300
Nitrophenol					< 2000	< 2300
benzofur <b>en</b>			**	**	< 120	< 140
4-Dinitrotoluene					< 120	< 140
etrylphthalate	50000	10000000	10000000		< 120	< 140
Chlorophenyl phonyl einer	30000	10000000	1000000			
	100000				< 120	< 140
uorene	100000	10000000	2500000		< 120	< 140
Niroaniline	T.	•			< 2000	< 2300
8-Dinivo-o-cresol					< 2000	< 2300
Nitrosociphenylamine	100000	600000	140000		< 120	< 140
Bromophenyl phenyl ether	and the second self-				< 120	< 140
exachiorobenzane	100000	2000	• 660		< 120	< 140
entechlorophanol	100000	24000	6000		< 2000	< 2300
nenanthrone .					210	< 140
ihracene	100000	10000000	10000000		91 _	< 140
arbazole	1			}	39 J	< 140
n-butyl phthelets	100000	10000000	5700000			1
	100000					< 140
uoran <b>thens</b>		100000 <b>00</b>	2300000		460	50 J
rene	100000	10000000	1700000		410	44 1
ityl benzyl phinalate	100000	10000000	1100000		< 120	< 140
3-Dichlorobenzidine	100000	e000	2000		< 120	< 140
enzo(a)anthracone	500000	4000	900	· .	280	< 140
rysene	500000	40000	9000		280	< 140
(2-ethylnexyl)phthalale (BEHP)	100000	210000	490 <b>00</b>		•	
					100 J	< 140
n-octyl phthalate	100000	10000000	1100000		< 120	< 140
nzo(b)fiuoranthene	50000	4000	900		450	41 ]
nzo(k)fluorenthene	500000	4000	900		190	< 140
nzo(a)pyrene	100000	€80	e6 <b>0</b>		360	< 140
leno(1,2,3-cd)pyrene	500000	4000	900		190	< 140
penzo(a,h)anihracene	100000	660	660		< 120	< 140
					- 16M	- ITU

<sup>[] -</sup> Indicates sample concentration greater than New Jersey Residential Soli Cleanup Criteria.

J - Estimated concentration less than the method detection limit.

D - Sample concentration determined by analysis of diluted sample.

E - Estimated minimum value, concentration greater than instrument calibration range.

TABLE 3-2

#### SOIL ANALYTICAL RESULTS SEMI - VOLATILE ORGANIC COMPOUNDS STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY

N. 144								
NEW ASSESSMENTS	The second second	HARRICAN THE MARK	Sales Land Control of March	のかは、東京の大学の大学の	のがなけいラーを呼ばない		200 May 14 (10)	
JERSEY TO THE REST OF THE PARTY	SITE-	SC MW 16L	SC-MW-16L SE	SC-MW-171 37	SC-MW-17L	SC-SB15	SC-SB18	SC SB15 💢
The second of th	1	A 10 10 10 10 10 10 10 10 10 10 10 10 10	The second of th		The contract of the second of	171490	1/14/89	inuss in the
RESIDENTIAL	DATE	: 1U300	אין פעבונו	Anama Table	1/12/99	profession and the second professional	No.	
DURECT CONTACT	DEPTH (n): TXX	1002 17 CAR	8-12	0-2	10-12	~ 0.2	0.2	9-10 to text (1) (20)
SOIL CLEANUP CRITICALA		TERES AND A CONTROL		E127/2017 STATE OF THE STATE OF	<b>全年的</b> 中国4000年		DILUTED TO	
BOC COSTON			<u> </u>					
10000000		< 380	< 440	500	< 780	< 410	< 16000 1 15 E	< 1300
		< 120	< 140	< 130		< 120	< 50 <b>00</b>	< 400
660					< 220	and the same		
280000		<b> &lt; 380</b>	< 440	< 430	< 790	< 410	< 18 <b>000</b>	< 1300
5100000		75 J	540	52 J	94 J	< 120 A STATE OF THE PROPERTY	< 5000	< 400 ·
570000		< 120	20 <b>00</b>	130 J	200 J	< 120	< 5000	< 400
	1	1 .			1	4.00		1.0
5100000		76 J	190	< 130	< 220	< 120	< 5000	< 400
2800000		< 380	< 440	230 J	< 730	< 410	< 16000	< 1300
	1	< 120	< 140	< 130	< 220	< 120	< 50 <b>00</b>	< 400
1 11 20 20 20 20 20 20 20 20 20 20 20 20 20				1				
2800000		< 380	< 440	< 430	< 730	< 410	< 16000	< 1300
660		< 120	< 140	< 130	< 220	< 120	< 50 <b>00</b>	< 400
6000		< 120	< 140	< 130	< 220	< 120	< 5000	< 400
		1	b .					3 1 1
28000		< 120	< 140	< 130	< 220	< 120	< 5000	< 400
1100000		< 120	< 140	< 130	< 220	< 120	< 5000	< 400
		< 380	< 440	< 430	< 730	< 410	< 16000	< 1300
1	J		_				10.00	
1100000	1.	< 380	< 440	220 J	< 730	< 410	< 16000	< 1300
1	I	< 120	< 140	< 130	< 220	< 120	< 5000	< 400
170000	1	< 380	< 440	< 430	< 730	< 410	< 16000	< 1300
	J			• • • • • • • • • • • • • • • • • • • •				and the second s
68000	1	< 120	< 140	< 130	< 220	< 120	< 5000	< 400
1 230000	1	< 120	< 140	40 J	92 J	210	< 5000	< 400
230000	1	< 120	< 140	< 130	< 220	< 120	< 5000	< 400
•		< 120	< 140				2. 4.34.2	7
1000	1			< 130	< 220	< 120	< 5000	< 400
10000000		< 120	< 140	< 130	< 220	< 120 · · ·	< 5000	< 400
		< 120	< 140	< 130	< 220	110 J	< 5000	< 400
400000		< 120	< 140	< 130	< 220	< 120	< 5000	< 400
						1		7.7
62000		< 380	< 440	< 430	< 730.	< 410	< 160 <b>00</b>	< 1300
5800000		< 2000	< 2300	l≺ 2200	< 3800	< 2100	< 64000 ····	< 6800
	1	< 120	< 140	< 130	< 220	< 120	< 5000	< 400
1		< 2000	-	•				
			< 2300	< 2200	< 3800	< 2100	< 84 <b>000</b>	< 6800
10000000		< 120	< 140	< 130	< 220	< 120	< 5000	< 400
		62 J	< 140	< 130	(89)	130	< 5000	< 400
		< 120	< 140	< 130				
				7	< 220	< 120	< 5000	< 400
		< 2000	< 2300	< 2200	< 3800 ⋅	< 2100	< 64000	< 6800
3400000	' '	< 120	< 140	< 120	520	240	< 5000	< 400
110000		< 2000	< 2300	< 2200	< 3800	< 2100	< 84000	
	i i	< 2000	< 2300				1	< 6800
	1	1		< 2200	< 3800	< 2100	< 84000	< 6800
		< 120	< 140	< 130	67 J	180	< 5000	< 400
1		< 120	<"140	< 130 .	< 220	< 120	< 5000	< 400
10000000		'	< 140	< 130	< 220			
	1					< 120	< 50 <b>00</b>	< 400
			< 140	< 130	< 220	< 120	< 50 <b>00</b>	< 400
2300000	l., , , , ,		< 140	< 130	170 J	240	< 50 <b>00</b>	< 400
1		< 20 <b>00</b>	< 2300	< 2200	< 3800	< 2100	<.84000	< 6800
1			< 2300	< 2200		1		
140000		l ,	< 140		< 3800	< 2100	< 84 <b>000</b>	< 6800
1-2000				150	< 220	< 120	< 50 <b>00</b>	< 400
<u> </u>	1		< 140	< 130	< 220	< 120	< 5000	< 400
660		< 120	< 140	< 130	< 220	< 120	< 5000	
6000			< 2300	< 2200			1	< 400
1		0.0			< 3800	< 2100	< 840 <b>00</b>	< 6800
		210	< 140	170	820	2000	2400 JD	230 J
10000000	4 1	91 J	< 140	58 J	280	570	< 50 <b>00</b>	< 400
	1	39 J	< 140	< 130	100 J			
5700000			< 140			360	< 50 <b>00</b>	< 400
2300000		A Company of the Comp		< 130	< 220	< 120	< 5000	< 400
		480	50 J	360	1200	3100	3500 JD	400
1700000		410	44 J	340	1200	6900 €	1	
1100000		< 120	< 140	< 130	•		2500 JD	320 J
2000	1				< 220	< 120	< 5000	< 400
	· •		< 140	< 130	< 220	< 120	< 5000	< 400
900	1	280	< 140	240	470	[1500]	1	1
9000			< 140	260			[1800] JD	200 J
49000	, '' <b> </b>				480	64 <b>00</b> E	1600 JD	200 J
-2000	ľ		< 140	75 J	< 220	[120000]. E	[330000] D	280 J
1100000	·		< 140	< 130	< 220	[190000] E	1	
900		450	41 J	480	1			< 400
900	·		< 140	_	680	[2200]	[2100] JO	300 J
			. 1	170	85 J	[2200]	< 5000	< 400
660			< 140	380	420	< 120	< 5000	
900	1.	190	140	< 130	< 220			210: J
660						860	< 5000	< 400
1		/		< 130	< 220	450	< 5000	< 400
		48 J .	140	210	160 J	200	< 500 <b>0</b>	< 400
24							1 - 5000	~~UU . '

SULTS COMPOUNDS CAL COMPANY

								to serve of the design of
1,42		学の対象の記述		19 (N 19			EC SB10	SC-9816
-2.2	SC MW-17L	SC-SB15	SC-SB15	SC-SB15	5C-SB18.	SC-SB16	1/14/99	1/1/99
	יוועבולוי.	0.2	0-2	1/14/99 8-10	18-18	0.2	10 12	18-18
	4:10:12		POLUTED					
14.2 2.3	* 2 . 4.44 . 200 * * * 100 / 100 / 2 + 4	7 (A) 4 (A) 4 (A)	1.0		-			< 410
- 1	< 730	< 410	< 16000 < 5000	< 1300 < 400	< 500 < 150	< 1600 < 470	< 460 < 140	< 120
	< 220	< 120 < 410	< 16000	< 1300	< 500	< 1600	< 460	< 410
	< 730 194 J	< 120	< 5000	< 400	< 150	< 470	< 140	< 120
1.	200 J	< 120	< 5000	< 400	< 150	< 470	< 140	< 120
. **	< 220	< 120	< 50 <b>00</b>	< 400	< 150	< 470	< 140	< 120
	< 730	< 410	< 16000	< 1300	< 500	< 1600	< 460	< 410
	< 220	< 120	< 5000	< 400	< 150	< 470	< 140 < 460	< 120 < 410
. ^ '	< 730	< 410	< 16000	< 1300 < 400	< 500 < 150	320 J < 470	< 140	< 120
	< 220 < 220	< 120 < 120	< 5000 < 5000	< 400	< 150	< 470	< 140	< 120
	< 220	< 120	< 5000	< 400	< 150	< 470	< 140	< 120
	< 220	< 120	< 50 <b>00</b>	< 400	< 150	< 470	< 140	< 120
	< 730	< 410	< 16000	< 1300	< 500	< 1600	< 460	< 410
٠.,	< 730	< 410	< 16000	< 1300	< 500	360 J	< 460	< 410
	< 220	< 120	< 5000	< 400 < 1300	< 150	< 470	< 140 < 460	< 120 < 410
	< 730	< 410	< 16000 < 5000	< 400	< 500 < 150	< 1600 < 470	< 140	< 120
-	< 220 92 J	< 120 210	< 5000	< 400	70	240 J	< 140	< 120
	< 220	< 120	< 5000	< 400	< 150	< 470	< 140	< 120
	< 220	< 120	< 5000	< 400	< 150	< 470	< 140	< 120
	< 220	< 120	< 5000	< 400	< 150	< 470	< 140	< 120
	< 220	110 J	< 5000	< 400	< 150	< 470	< 140	< 120 < 120
.	< 220	< 120	< 50 <b>00</b> < 16 <b>000</b>	< 400 < 1300	< 150 < 500	< 470 < 1600	< 140 < 460	< 120 < 410
	< 730 < 3800	< 410 < 2100	< 84000	< 6800	< 2800	< 8100	< 2300	< 2100
	< 220	< 120	< 5000	< 400	< 150	< 470	< 140	< 120
'	< 3800	< 2100	< 84000	< 8800	< 2800	< 8100	< 2300	< 2100
_	< 220	< 120	< 5000	< 400	< 150	620	< 140	< 120
	(66)	130	< 5000	< 400	< 150	< 470	< 140	< 120 < 120
	220	< 120	< 5000	< 400 < 6800	< 150 < 2800	< 470 < 8100	< 140 < 2300	< 120 < 2100
	< 3800 5 <b>20</b>	< 2100 240	< 84000 < 5000	< 400	< 150	< 470	< 140	< 120
	< 3800	< 2100	< 84000	< 8800	< 2800	< 8100	< 2300	< 2100
	< 3800	< 2100	< 84000	< 6800	< 2600	< 8100	< 2300	< 2100
	67 J	160	< 50 <b>00</b>	< 400	< 150	< 470	< 140	< 120
	< 220	< 120	< 5000	< 400	< 150	< 470	< 140	< 120 < 120
	< 220	< 120	< 5000	< 400 < 400	< 150 < 150	< 470	< 140 < 140	< 120
	< 220 170 J	< 120 240	< 5000	< 400	< 150	< 470	< 140	< 120
	≺ 38 <b>00</b>	< 2100	< 84000	< 6800	< 2800	< 8100	< 2300	< 2100
	< 3800	< 2100	< 840 <b>00</b>	< 6800	< 2600	< 8100	≺ 2300	< 2100
	< 220	< 120	< 50 <b>00</b>	< 400	< 150	< 470	< 140	< 120
٠.	< 220	< 120	< 5000 /.	< 400 < 400	< 150 < 150	< 470 < 470	< 140 < 140	< 120 < 120
	< 220	< 120 < 2100	< 5000 < 84000	< 6800	< 2600	< 8100	< 2300	< 2100
	< 3800 820	2000	2400 JD	230 」	< 150	< 470	< 140	< 120
.	280	570	< 5000	< 400	< 150	< 470	< 140	4 120
	100 J	360	< 50 <b>00</b>	< 400	< 150	< 470	< 140	< 120
	< 220	< 120	< 5000	< 400	< 150	< 470	< 140	< 120 < 120
	1200	3100	3500 JD 2500 JD	400 320 J	< 150 < 150	< 470 < 470	< 140 < 140	< 120
	1200	6900 €	2500 JD < 5000	< 400 J	< 150	< 470	< 140	< 120
	< 220 < 220	< 120 < 120	< 5000	< 400	< 150	< 470	< 140	< 120
;	470	[1508]	[1800] JD	200 J	< 150	< 470	< 140	< 120
;	480	6400 E	1600 JD	200 J	< 150	< 470	<. 140	< 120
	< 220	[120000] E	[220000] D	290 J	71 J	< 470	< 140	< 120
	< 220	[190000] E	[190000] D	< 400	< 150	< 470	< 140	< 120
	680	[2200]	[210 <b>0]</b> JD	200 J	< 150	€ 470	< 140	< 120
. 1	95 J	[2200]	< 5000	< 400	< 150	< 470	< 140	< 120
	420	< 120	< 5000	210 J	< 150	< 470	< 140	< 120 < 120
	< 220	960	< 50 <b>00</b> < 50 <b>00</b>	< 400 < 400	< 150 < 150	< 470 < 470	< 140	< 120
- 1	< 220 -	450 200	< 5000 < 5000	< 400	< 150	< 470	< 140	< 120
لـــ	100 J					<del></del>		

TABLE 3-3

### GROUNDWATER ANALYTICAL RESULTS STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY

	NEW JERSEY	SITE:	SC-MW-16L	SC-MW-17L	SC-MW-17L
	CLASS D-A	DATE:	2/2/99	2/2/99	2/2/99
	GROUNDWATER				
CONSTITUENT:	QUALITY				DILUTED
(Units in ug/l)	CRITERIA	最高数据 2 MP 185 年			SAMPLE
Benzo(a)pyre <b>ne</b>	0.003		< 50	< 10	< 1000
2,4-Dinitrophenol	10		< 250	< 50	< 5000
Dibenz(a,h)anthracene	0.003		< 50	< 10	< 100 <b>0</b>
Benzo(a)anthracene	0.03		< 50	< 10	< 1000
4-Chloro-3-methylphenol			< 50	< 10	< 1000
Hexachloroethane	0.7	· ·	< 50	< 10	< 1000
Hexachlorocyclopentadiene	50		< 50	< 10	< 1000
Isophorone	100		< 50	< 10	< 1000
Acenaphthene	400		< 50	< 10	< 1000
Diethylphthalate	5000		< 50	< 10	< 1000
Di-n-butylphthalate	900		< 50	< 10	< 1000
Phenanthrene		1.	< 50	< 10	< 1000
Butyl benzyl phthalate	100		< 50	< 10	< 1000
N-Nitrosodiphenylamine	7		< 50	< 10	< 1000
Fluorene	300	- ·	< 50	< 10	< 1000
Carbazole			< 50	< 10	< 1000
Hexachlorobutadiene	1		< 50	< 10	< 1000
Pentachlorophenol	0.3		< 250	< 50	< 5000
2,4,6-Trichlorophenol	3		< 50	< 10	< 1000
2-Nitroaniline		*	< 250	< 50	< 5000
2-Nitrophenol			< 50	< 10	< 1000
Naphthalene			6 J	12	< 1000
2-Methylnaphthalene		-	< 50	< 10	< 1000
2-Chloronaphthalene			< 50	< 10	< 1000
3,3'-Dichlorobenzidine	0.08		< 50	< 10	< 1000
2-Methylphe <b>nol</b>			< 50	< 10	< 1000
1,2-Dichlorobenzene	600		33 <b>0</b>	[3800] E	[12000] D
2-Chlorophenol	40		< 50	25	< 1000
2,4,5-Trichlorophenol	700		< 250	< 50	< 50 <b>00</b>
Nitrobenzen <b>e</b>	3		< 50	< 10	< 1000
3-Nitroaniline			< 250	< 50	< 5000
4-Nitroaniline			< 250	< 50	< 5000
4-Nitrophenol		`	< 250	< 50	< 5000
4-Bromophenyl phenyl ether			< 50	< 10	< 1000
2,4-Dimethylphenol	100		< 50	< 10	< 1000
4-Methylphenol			< 50	< 10	< 1000
1,4-Dichlorobenzene	75		[540]	[3500] E	[11000] D
4-Chloroaniline			< 50	< 10	< 1000
Phenol	4000		< 50	< 10	< 1000
Bis(2-chloroethyl)ether	0.03		< 50	< 10	< 1000
Bis(2-chloroethoxy) methane		]	< 50	< 10	< 1000
Bis(2-ethylhexyl)phthalate	3		< 50	< 10	< 1000
Di-n-octylphthalate	100		< 50	< 10	< 1000
Hexachlorobenzene	0.02		< 50	< 10	< 1000
Anthrace <b>ne</b>	2000		< 50	< 10	< 1000
1,2,4-Trichlorobenzene	2000		< 50	[17]	< 1000
2,4-Dichlorophenol	20			l	< 1000
2,4-Dinitrotoluene	20 0.05		< 50 < 50	6 J < 10	< 1000
Pyren <b>e</b>	200		< 50	< 10	< 1000

#### TABLE 3-3

### GROUNDWATER ANALYTICAL RESULTS STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY

CONSTITUENT: (Units in ug/l)	NEW JERSEY CLASS II-A GROUNDWATER CUALITY CRITERIA	SITE: DATE:	SC-MW-16L 2/2/99	SC-MW-17L 2/2/99	SC-MW-17L 2/2/99 DILUTED SAMPLE
Dimethylphthalate	7000		< 50	< 10	< 1000
Dibenzofur <b>an</b>			< 50	< 10	< 1000
Benzo(g,h,i)perylene			< 50	< 10	< 1000
Indeno(1,2,3-cd)pyrene	0.03		< 50	< 10	< 1000
Benzo(b)fluoranthene	0.03		< 50	< 10	< 1000
Fluoranthene	300		< 50	< 10	< 1000
Benzo(k)fluoranthene	0.03	and the second	< 50	< 10	< 1000
Acenaphthylene			< 50	< 10	< 1000
Chrysene 2	0.03	17	< 50	< 10	< 1000
4,6-Dinitro-2-methylphenol		•	< 250	< 50	< 5000
1,3-Dichlorobenzene	600		460	[4100] E	[7400] D
2,6-Dinitrotoluene		•	< 50	< 10	< 1000
N-Nitrosodi-n-propylamine	0.005		< 50	< 10	< 1000
4-Chlorophenyl phenyl ether			< 50	< 10	< 1000
2.2'-oxybis(1-chloropropane)			< 50	< 1.0	< 1000

- [ ] Indicates sample concentration greater than New Jersey Class II-A Groundwater Quality Criteria.
- J Estimated concentration less than the method detection limit.
- D Sample concentration determined by analysis of diluted sample.
- E Estimated minimum value, concentration greater than instrument calibration range.

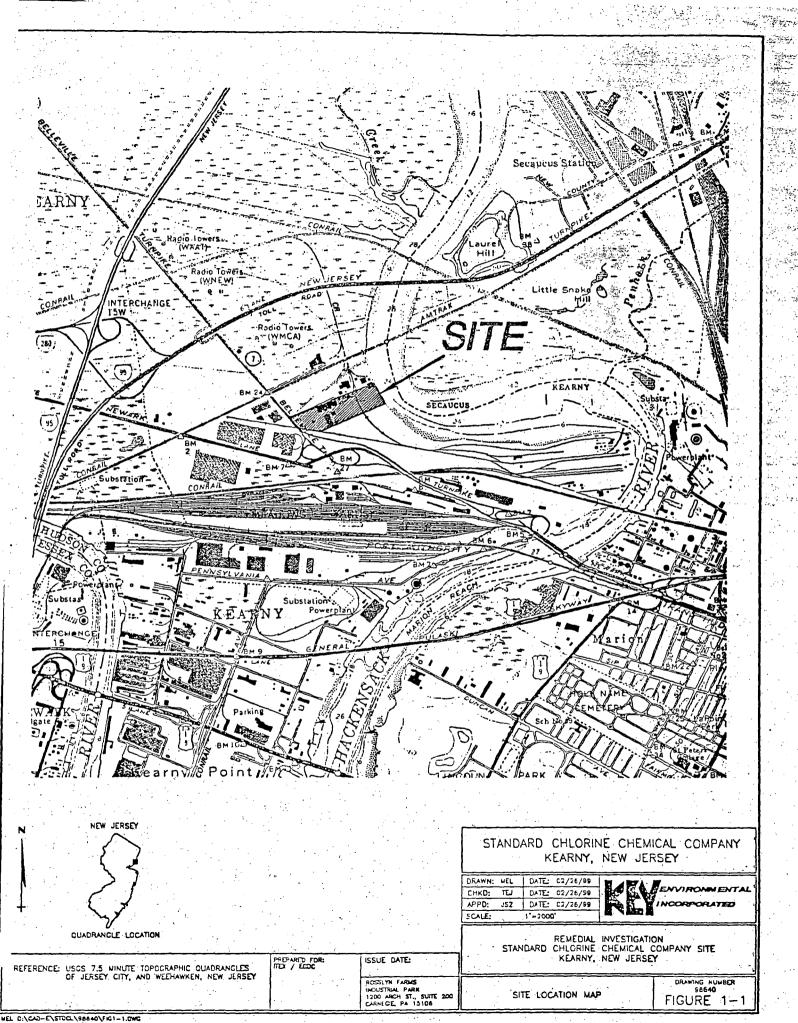
**TABLE 3-4** 

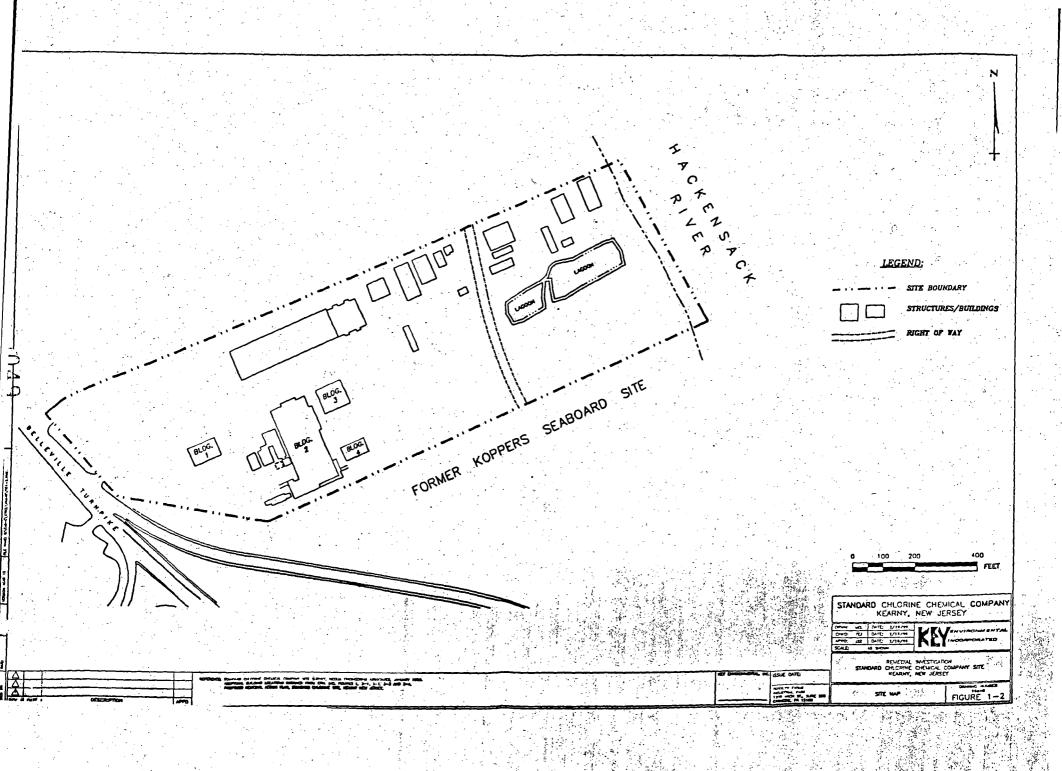
## POTENTIOMETRIC SURFACE MEASUREMENTS STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY

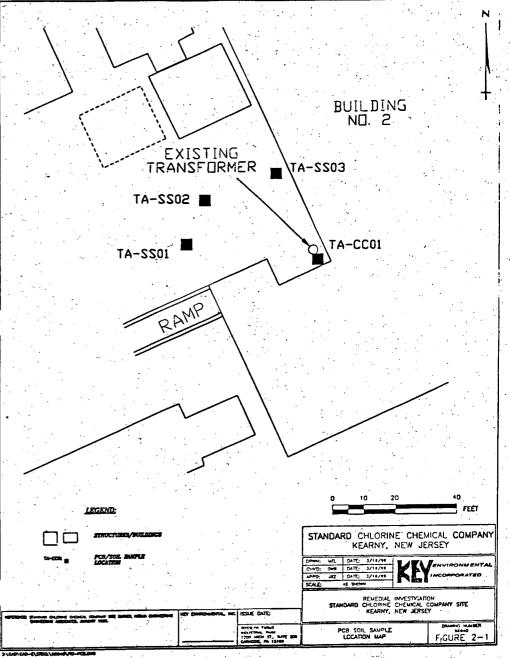
		MEASURING.	DEPTH TO	POTENTIOMETRIC
DATE	MONITORING	POINT	GROUND	SURFACE
	WELL	ELEVATION	WATER	ELEVATION
		(ft-msl)	(FT)	✓ (ft-msl)
2/2/99	MW-1L	8.54	5.1	3.44
2/2/99	MW-2L	7.36	4.06	3.3
2/2/99	MW-3L	5.29	2.1	3.19
2/2/99	MW-4L	7.28	4.34	2.94
2/2/99	MW-5L	6.14	2.65	3.49
2/2/99	MW-6L	6.82	3.56	3.26
2/2/99	MW-7L	6.9	3.26	3.64
2/2/99	MW-8L	8.5 <b>8</b>	6.59	1.99
2/2/99	MW-9L	10.09	8.55	1.54
2/2/99	MW-10L	8.12	5.69	2.43
2/2/99	MW-11L	7.88	4.18	3.7
2/2/99	MW-11U	7.83	3.16	4.67
2/2/99	MW-12L	6.9 <b>9</b>	3.65	3.34
2/2/99	MW-12 <b>Ú</b>	8. <b>33</b>	4.19	4.14
2/2/99	MW-13L	11.61	8.92	2.69
2/2/99	MW-13 <b>U</b>	11.26	6.5 <b>9</b>	4.67
2/2/9 <b>9</b>	MW-14L	7.9 <b>9</b>	5.37	2.62
2/2/9 <b>9</b>	MW-14U	8.8 <b>9</b>	3.79	5.1
2/2/99	MW-15L	6.3 <b>6</b>	3.21	3.15
2/2/9 <b>9</b>	MW-15U	6.44	2.72	3.72
2/2/99	MW-16L	7.82	4.32	3.5
2/2/9 <b>9</b>	MW-17L	3.87	0.89	2.98
2/2/99	MW-107	4.24	0.54	3.7
2/2/99	MW-119	4.2	0	4.2
2/2/99	PZ-2U	7.6	5.2	2.4
2/2/99	PZ-4U	7.2	3.75	3.45
2/2/99	PZ-5U	10.92	7.9 <b>8</b>	2.94

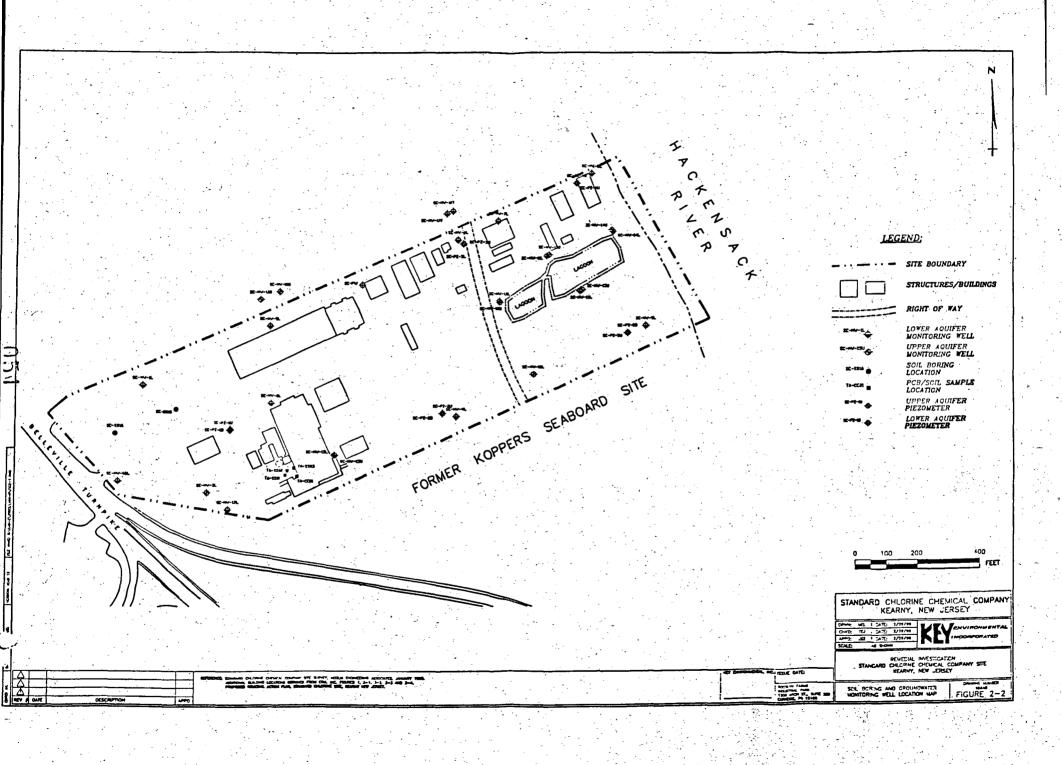
Elevations are in feet above mean sea level referenced to the North American Vertical Datum of 1988.

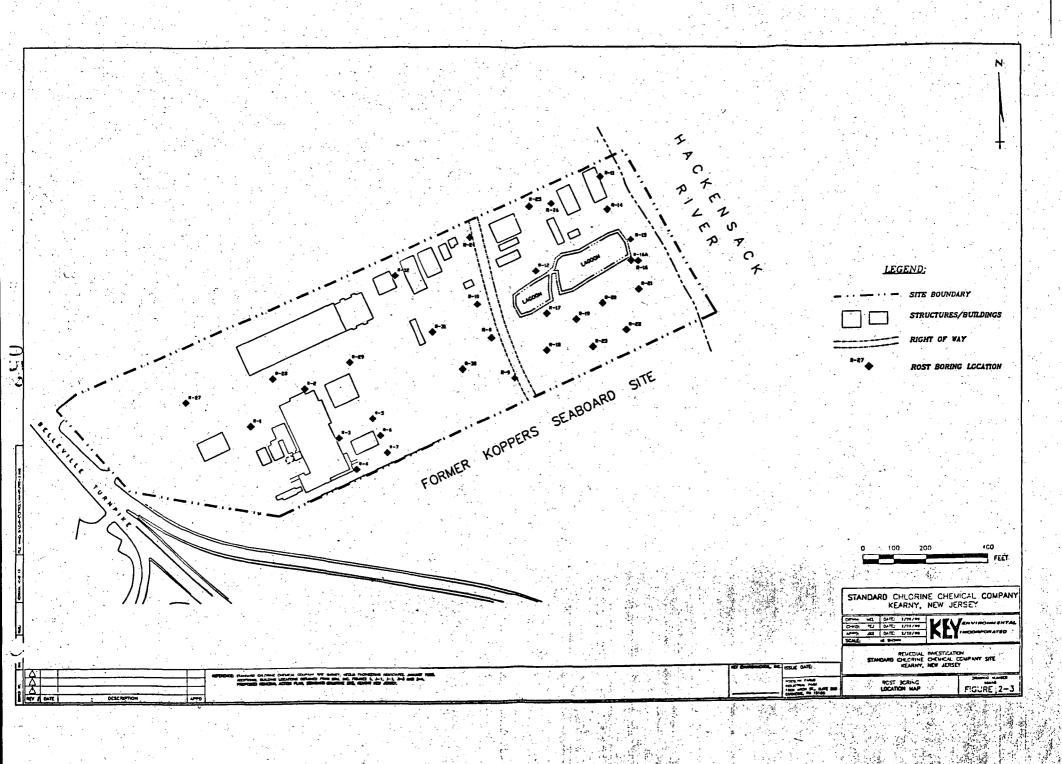
#### **FIGURES**



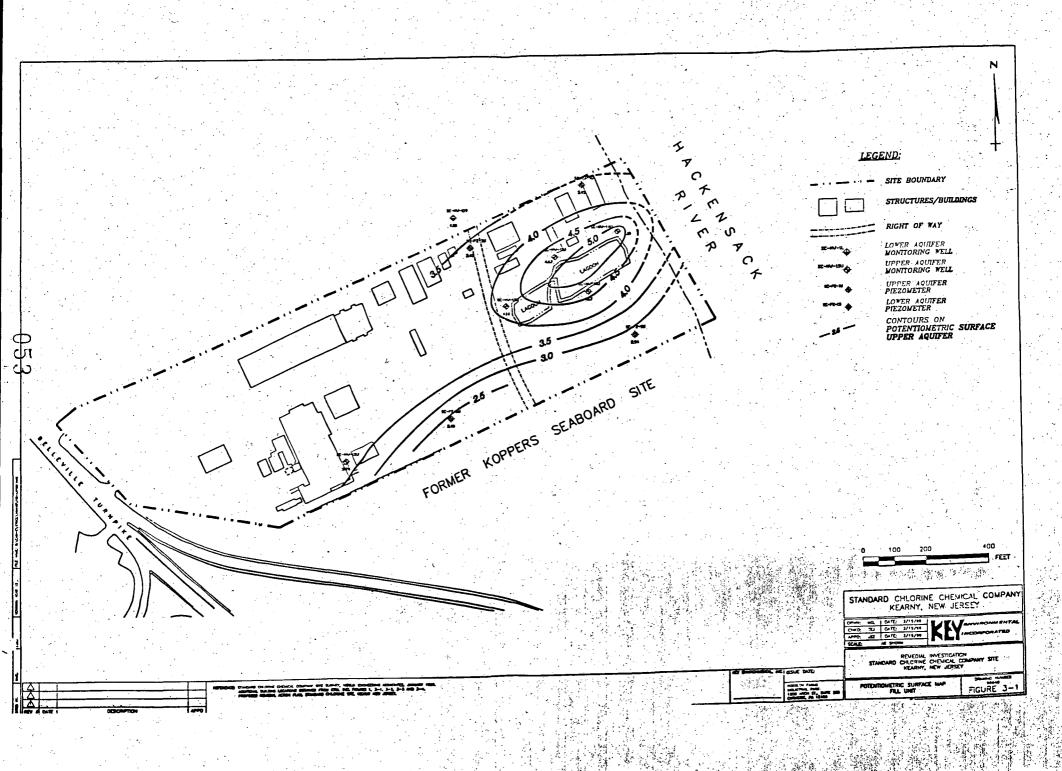


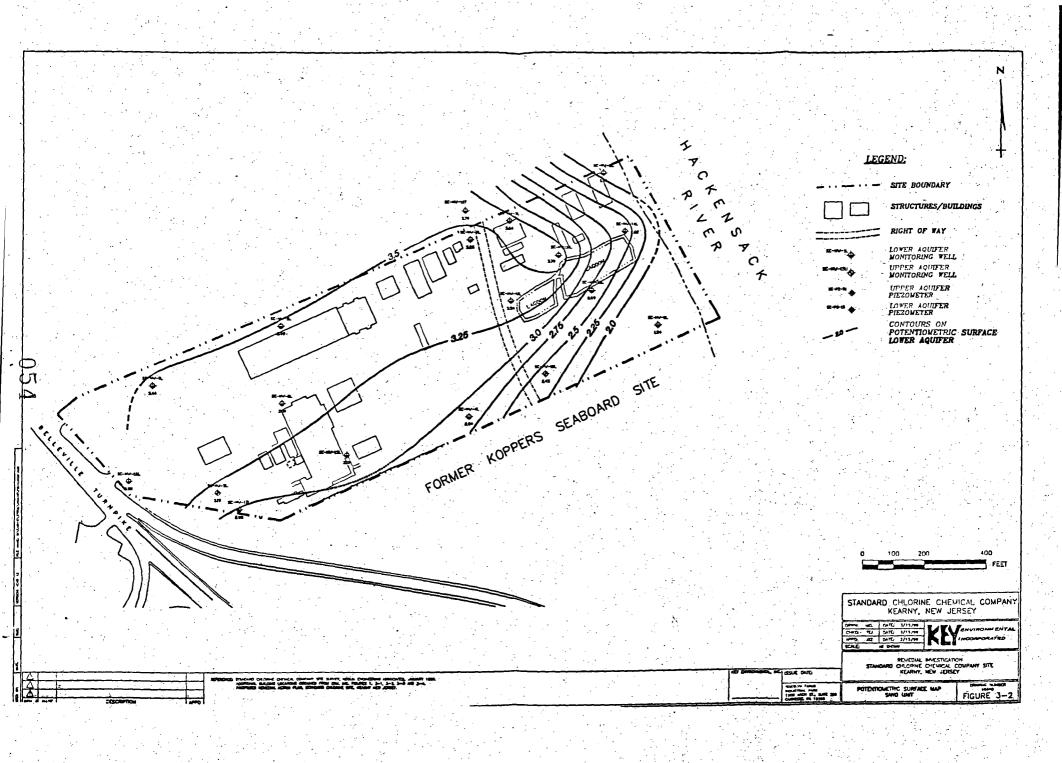


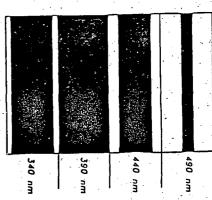




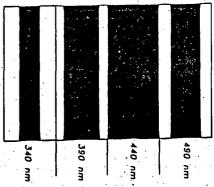
1. 1. 18 S. 18 16







TYPE #1 NAPL SIGNATURE



TYPE #2 NAPL SIGNATURE

NOTES: PRODUCT IDENTIFICATION BASED ON RELATIVE FLUORESCENCE INTENSITY AS INDICATED BY THE BANDWIDTHS.

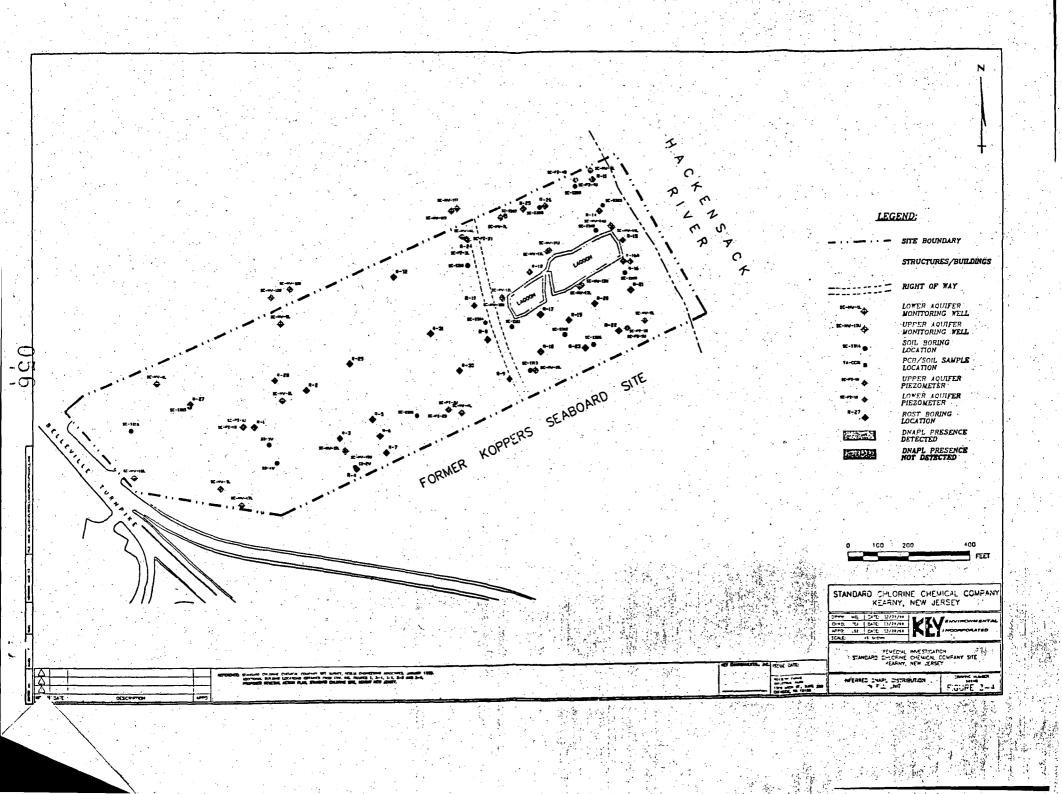
> MORNE MICHISSUE CATE HOTS TO FAME I TOO MICH ST., SUITE 200 CAMECE, IN 15100

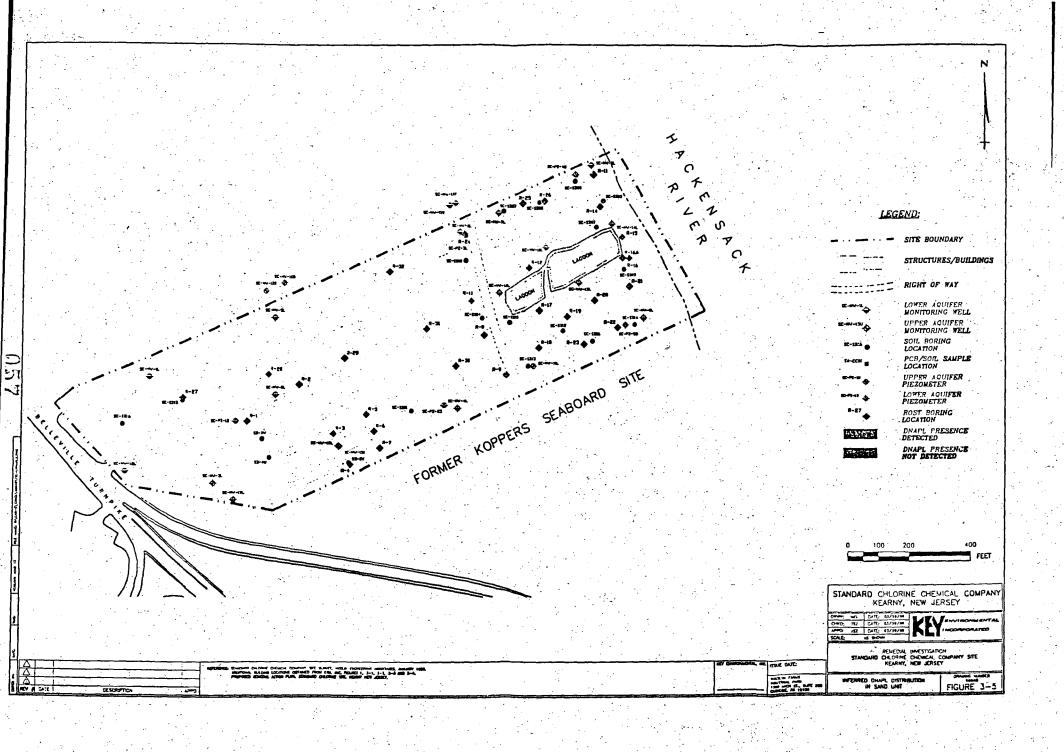
STANDARD CHLORINE CHEMICAL COMPANY KEARNY, NEW JERSEY CHANGE MEL CATE 03/29/49
CHAO: TEJ CATE 03/29/49
APPO: AN CATE 03/29/49
IMPORTANTED

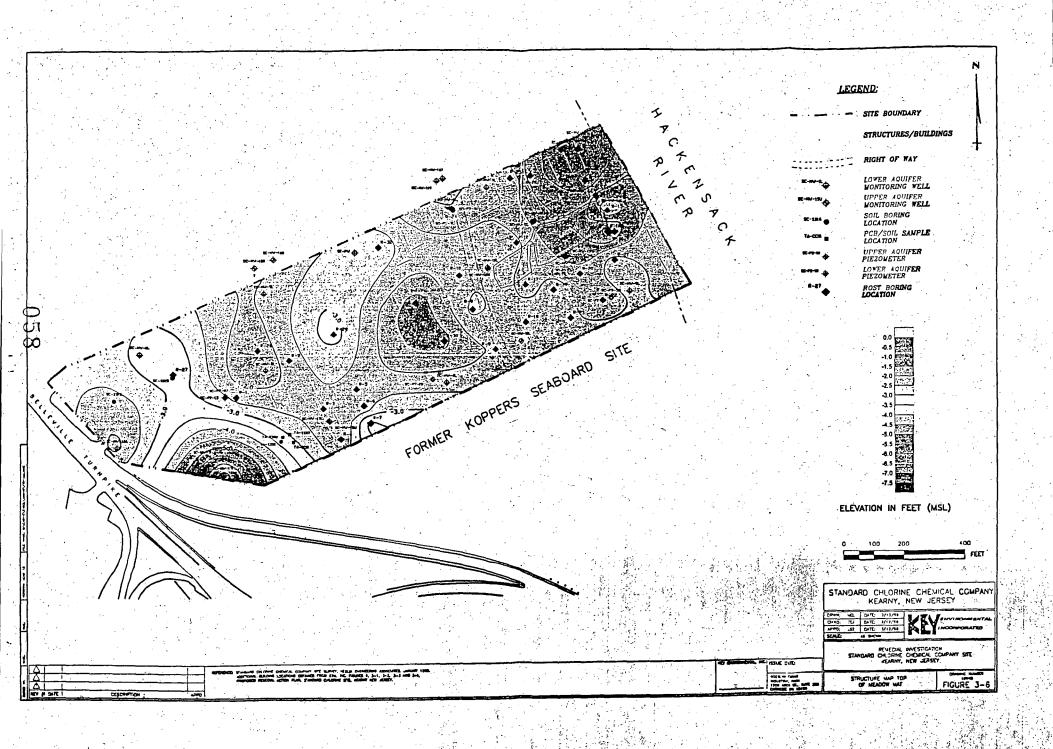
RENECIAL INVESTIGATION
STANDARD CHLORINE CHEMICAL COMPANY SITE
KEARNY, NEW JERSEY

TYPICAL MULTI-WAVELENGTH (NWL)

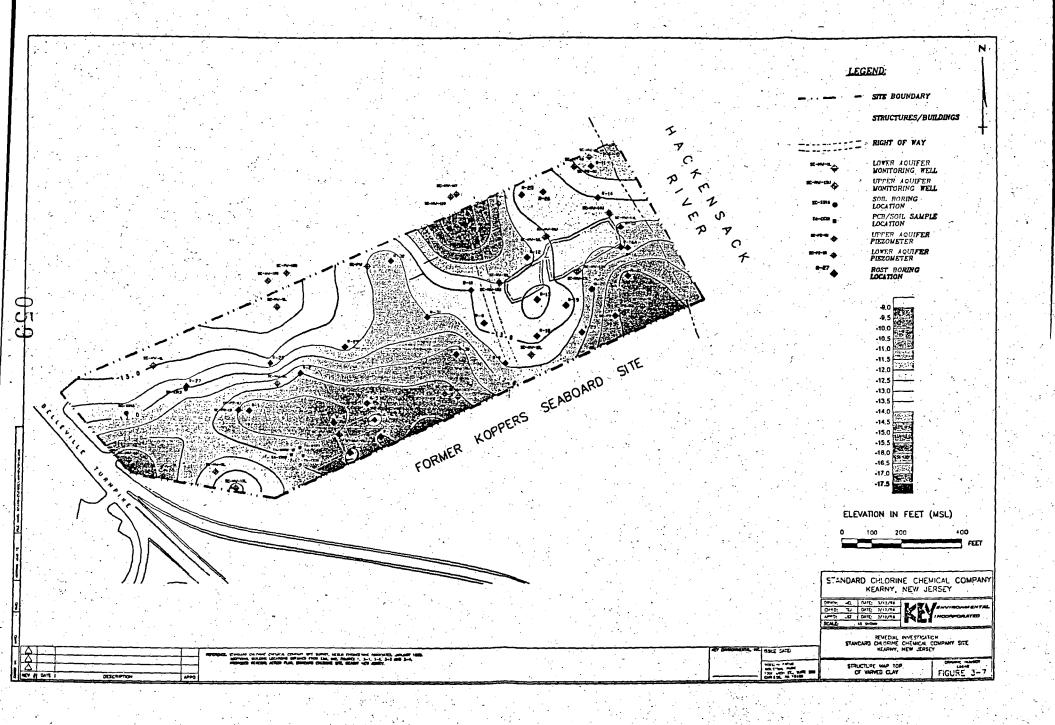
F GURE 3-3

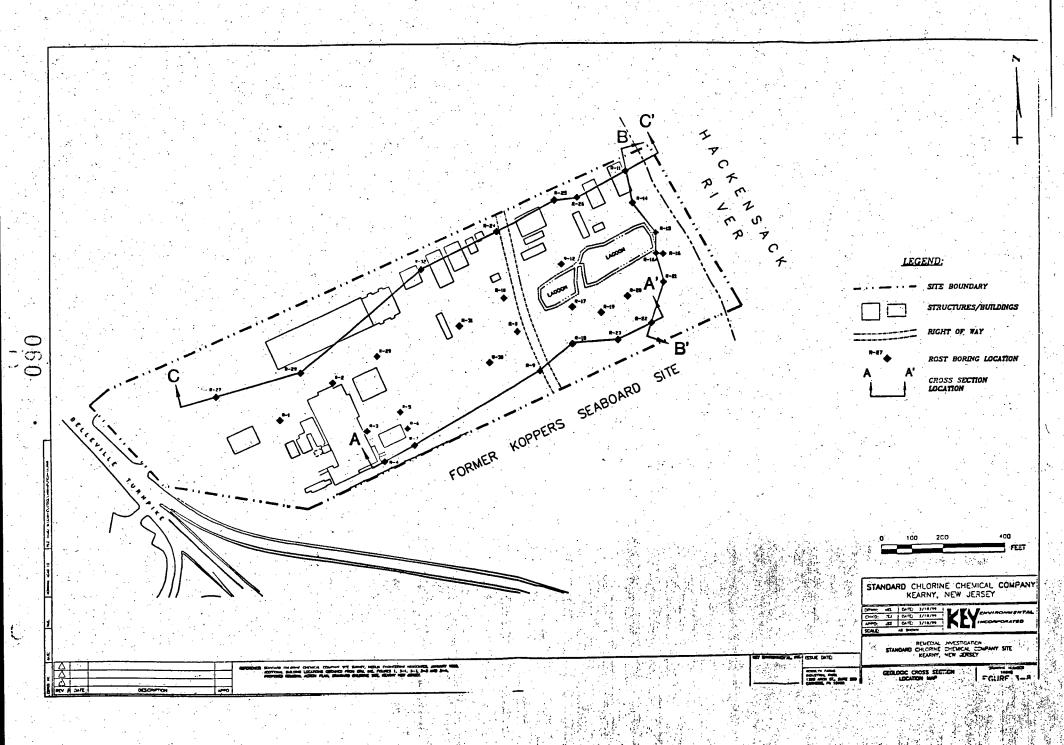






to A Decide No. Tool Sea

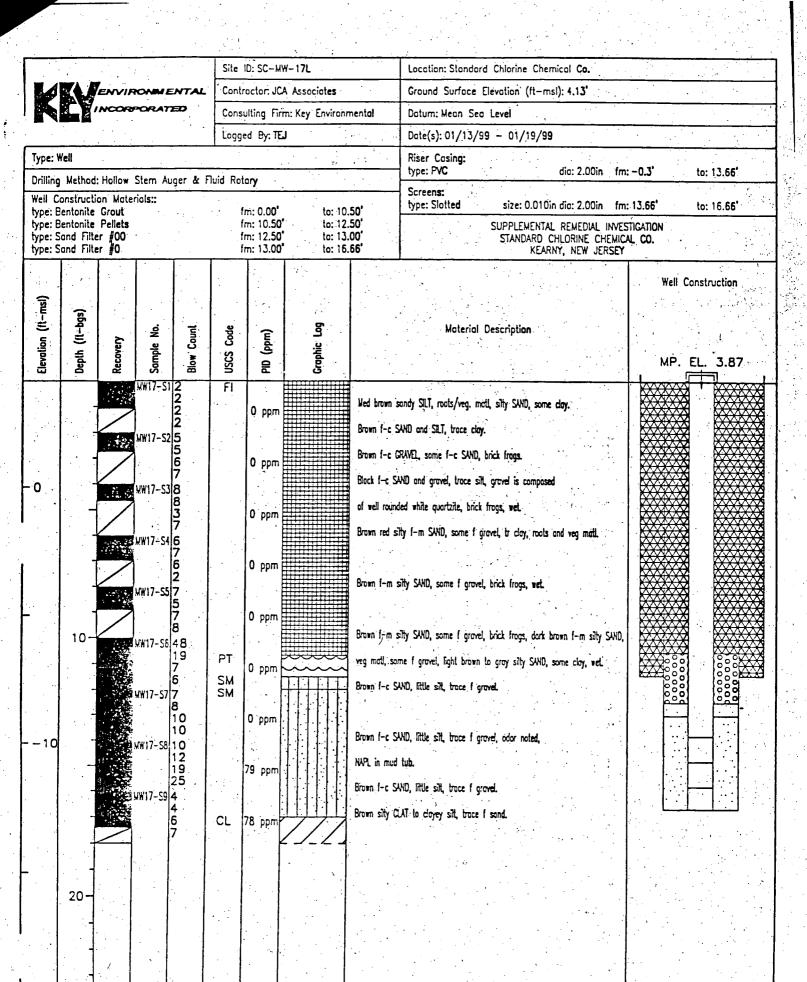




#### APPENDIX A

BORING LOGS AND MONITORING WELL CONSTRUCTION DIAGRAMS

			*				•			
		<u>.</u>			Sile	ID: SC-M	W_ 16I		Location: Standard Chlorine Chemical Co.	
	eya <i>a</i>	FNVIR	NONME	ENTAL	·		A Associotes	<u> </u>	Ground Surface Elevation (ft-mst): 8.00'	
K		NCOR					m: Key Environn	nental	Datum: Mean Sea Level	
<b>M V</b>		7 . 7			<u> </u>	ed By: TE			Dote(s): 01/13/99 - 01/18/99	
Type: \	Yell	•			1 1099			<del></del>	Riser Casing:	
	<del></del>	Hollow	Stem A	uger-&-F	Tuid Rot	0 <b>r</b> V			type: PVC dia: 2.00in fm	: -0.2' to: 13.20'
	onstructi	<del></del>		- y ·	٠. ,	<del></del>			Screens: type: Slotted size: 0.010in dia: 2.00in fm	: 13.17' to: 18.17'
type: E	lentonite lentonite and Filte lentonint	Peliets r			fi fi	m: 0.00' m: 10.40' m: 12.33' m: 18.17'	to: 18.	33' 17'	SUPPLEMENTAL REMEDIAL INV. STANDARD CHLORINE CHEMI KEARNY, NEW JERSE	ESTIGATION
Elevation (It-msl)	Depth (11-bgs)	Recovery	Sample No.	Blow Count	USCS Code	PiO (ppm)	Graphic Log		Material Description	Well Construction  MP. EL. 7.82
			16L- <b>S1</b>	3	FI			Dark brown	sondy SILT, roots/vegalative material	
1				7		0 ppm		Red brown s	alty f-m SAND, and f-m gravel, gloss frags, maist.	
-	_		16L-S2	3 3					ity f-m SANO, and f-m gravel, red sandstone rock frags, wet.	
:				3 5		0 ppm				
		- 14	16L-S <b>3</b>	5					AND, little f gravel, trace silt, block	
	-		<u> </u>	27 13		0 ppm			ing present along horizontal seams, wet.	
			16L-S4	3	· .			Red brown f	-c SAND, and gravel, some sitt, wet.	
	, -			4 2		0 ppm				
0	.		16L-S5	3				Brown f-m	SAND, some f grovel, little siit.	
	4			3 1		0 ppm		Black sandy	SILT, with vegatative most present (meadow mat).	
	10-		, e, . ce	1	PT					
			16L-S6	3				Block sandy	SILT, with vegetelive mell present, (meedew mat).	
				4		0 ppm		, , ,		0000
			16L-S7	2	SW		)	Brown f-m	SAND, some silt, some f-m gravel.	
				7 8		0 ppm				
	-		16L-S8	9 12					SWD, some silt, some f-m grovel.	
				14		0 ppm		Gray brown f	-m SAND, little silt, troce f grovel.	
			16L-S <b>9</b>	11			, , ,	*		
<u>.</u>	4			18 15		0 ppm		Groy I-m St	ND, little sift.	
- 10				11.	ML.		· · · · · · · · · · · · · · · · · · ·			
			16L-S10	8	· IAIF	[		Red brown d	layey SILT, some f sand, varved.	60000000000000000000000000000000000000
						0 ppm[				
	20-				. :	<u> </u>				
	: :1			٠,	. ,					
	4									
	]					}				
-	ľ									



Control (Control (Con			Site ID: SC-SE	15		Location: Standard Chlorine Chemica				\ -\[`
Logged By: TEJ  Dote(s): 07/14/99  Per Soil Bothing  Per Soil Bothing  Ref Cosing:  NA  Supplication Moteriols:  Assume Construction Moteriols:  NA  Supplication	ENVIRON	MENTAL	Contractor: JC	Associates		<del></del>	5.37			
PRINT Method: Hollow Stem Auger  IIII Construction Meteriols:  A  SUPPLEMENTAL RELIGIOU. INVESTIGATION STANDARD CALIDRISE CHANCAL CO.  KEARNY, NEW AIRSEY  SIDES STANDARD CALIDRISE CHANCAL CO.  Molerial Description  Borehole Construction  Borehole Const	INCORPOR	RATEO	Consulting Fire	n: Key Environm	ental		* . * . * . *			∐։
A significant Meteriolas:  A Super to 1', brown sity SAND, if groved.  Significant from the significant of t	•		Logged By: TE.	<u> </u>						
Screens:  N/A  SUPPLEADING REMOVAL NEWSTRATION SINANDAD DISTORM CHEWICAL CO.  KELOW, N. W. WESTRATION SINANDAD DISTORM CHEWICAL  N. KELOW, N. W. WESTRATION SINANDAD DISTORM CHEWICAL  N. KELOW, N. W. WESTRATION SINANDAD DISTORM CHEWICAL  N. KELOW, N. W. WESTRATION  Red sarry SLT, some i grovel, most.  Red sarry SLT, sellow and block i-m grovel, welt.  Croy green SAND and SLT, some i grovel, yellow sity nodules.  O ppm  Croy green SAND and SLT, some i grovel, yellow sity nodules.  Croy green SAND and SLT, some i grovel, yellow sity nodules.  Croy green SAND and SLT, some i grovel, yellow sity nodules.  Croy green SAND and SLT, some i grovel, yellow sity nodules.  Croy green SAND and SLT, some i grovel, yellow sity nodules.  Croy green SAND and SLT, some i grovel, yellow sity nodules.  Croy green SAND and SLT, some i grovel, yellow sity nodules.  Croy green SAND, some sail, trace grovel.  Croy green SAND, some sail, trace grovel.  Croy stained block i-m SAND, some sail, trace grovel.  Croy stained block i-m SAND, some sail, trace grovel.  Red brown sardy SLT, trace clay.	pe: Soil Boring				•	Riser Cosing:				,   ?
A SUPPLIEDTAL GENERAL INVESTIGATION STANDARD CHIGHNEY CO. KENNY, NEW JESSEY  Moterial Description  Borehole Construction  Borehole Constr	illing Method: Hollow Ster	m Auger					· idita			
SSIS-SSIS-SSIS-SSIS-SSIS-SSIS-SSIS-SSI	ell Construction Materials	<b>::</b>				N/A				
SSI5-SI 5 1 2	<b>(X</b>					STANDARD CHLO	DRINE CHEMIC	AL CO.		
SSI5-SI 5 SSI5-SI 5 SSI5-SI 5 SSI5-SI 5 SSI5-SI 6 SSI5-SI 7 SSI5-S	Elevation (II-msi) Depth (II-bgs) Recovery			Graphic Log		Material Description		Boreho	le Construction	
Gray green f-c. SAND, some silt.  Gray green f-c. SAND, some silt.  Gray green f-c. SAND, some silt.  Gray green SAND and SILT, some f gravel, yellow silty nodules.  Gray green SAND and SILT, some f gravel, yellow silty nodules.  Gray green SAND and SILT, some f gravel, yellow silty nodules.  Dork brown SILT, some sond, vegalative motil present, H2S noted.  Gray green sandy SILT, grades to clayey SILT at base.  Gray green SAND, some silt, trace gravel.  Gray stained black f-m SAND, little silt, trace f gravel.  Gray stained black f-m SAND, little silt, trace f gravel.  Red brown sandy SILT, trace clay.		15-S1 5 12 15-S2 14 17 18	0 ppm		Red sondy	SILT, some f-m gravel, moist.  SILT, yellow and black f-m gravel, wet.				
SB15-SS 1 SB15-SS 3 SB15-S	SS 1	15-SJ 2 2 4 4 15-S4 5								
SB15-SB 3 SB15-SB 3 SB15-SB 3 SB15-SB 3 SB15-SB 3 SB15-SB 3 SB15-SB 11 SB15-S	SB	9	PT		Groy green	SAND and SILT, some 1 gravel, yellow s	silty nodules.			
SB15-S8 3 5 12 1.5 1.5 1.7 1.4 ML 0 ppm Cray stained black f-m SAND, little silt, trace f gravel.  Gray stained black f-m SAND, little silt, trace f gravel.  Gray stained black f-m SAND, little silt, trace f gravel.  Red brown sondy SiLT, trace clay.	10	3 3 6	0 ppm							
SB15-S9 1.1 1.5 1.7 1.4 ML 0 ppm Cray stained black f-m SAND, little silt, trace f gravel. Red brown sandy SILT, trace clay.	28	13					avel.			
Red brown sondy SILT, trace clay.	-10 SS	15-S9 1 1 1 5								
20-		14	ML U ppm		Red brown	sondy SILT, troce clay.				
	20-									

	T		
*** /***** ##	Site ID: SC-SB16	Location: Standard Chlorine Chemical Co.	
ENVIRONMENTAL	Contractor: JCA Associates	Ground Surface Elevation (ft-msl): 5.01'	
The state of the s	Consulting Firm: Key Environm	<del></del>	
	Logged By: TEJ	Date(s): 01/14/99 - 01/14/99	
Type: Soil Boring		Riser Casing:	
Drilling Method: Hollow Stem Auger		Screens:	
Well Construction Materials::		N/A	FERROLPOU
<b>N/A</b>		SUPPLEMENTAL REMEDIAL IM STANDARD CHLORINE CHEN KEARNY, NEW JERSI	IICAL CO.
(Ism-			
n (ft. y y y No. No.	ode (m)	Material Descripti <b>on</b>	Borehole Construction
Elevation (11-ms1) Depth (11-bgs) Recovery Sample No. Blow Count	USCS Code PID (ppm) Graphic Log		
B S B S	S   E   S FI   IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII		
		Auger 1° frazen gravel, brown gray f-m SAND, f gravel and green sand prese	nt.
SB16-S1 <b>5</b>	0 ppm	Brown gray f-m SAND, no green sond/gravel present, wet at 2'.	
S316-S2 3			
9	0 ppm		
SB16-S3 12		Brown gray f-c SAND, some f-m gravel, wet.	
-0 - 12	0 ppm		
SB16-S4 4		Red-brown f-c SAND, some f grovel, yellow staining on grovel.	
4		Dark brown sandy SLT, with vegatative most present.	
$\overline{3}$	PT 0 ppm		
SB16-S5 3 2		Dark brown sandy SLT, with regalative most present.	
	0 ppm		
- 10-SB16-S6 1		Dark brown sandy SILT, with regardative most present.	
3	ML 0 ppm	Gray green sondy SILT, some 1 sand, little clay.	
SB16-S7 6	SM		
10	0 ppm	Gray brown f-m SAND, some silt, grades to f-c SAND at base.	
15			
SB16-S8 2 7		Gray brown 1-c SAND, some f gravel.	
14	0 ppm	Gray brown f-c SAND, some f gravel.	
SB16-S9 9		Brown to red varved clayer SILT, some f sand.	
10 12 11	CL 0 ppm	were writer the stayed alles south a suite.	
- 20-			
		I rell	Pear 1 of 1
		nroot 154	Page 1 of 1